



CITY OF DOVER

PLANNING TECHNICAL REVIEW COMMITTEE –AGENDA

Meeting Type: Regular Meeting
Meeting Location: 1st Floor Conference Room – City Hall, Dover NH
Meeting Date: **Thursday, December 27, 2018**
Meeting Time: **10:30 am**

1. **10:30 AM – 11:15 AM:** Conditional Use Permit for Lilac Garden LLC, Assessor's Map H, Lot 35-D, zoned RM-SU, located at Lilac Lane. (Proposal is to construct a trash compactor area to support waste removal for the 50-acre existing housing complex. The construction consists of 235 sq. ft. of impervious area and 4,495 sq. ft. of disturbance within the 50 ft. wetland buffer). (P18-64)



CITY OF DOVER

TECHNICAL REVIEW COMMITTEE MINUTES – P18-57

Application Type: Site Plan Review
Applicant(s): DMC Real Estate Holdings, LLC
Owner(s): Kevin & Lydia Cooper
Location: 67 Knox Marsh Road (Tax Map H, Lot 33)
Date: November 29, 2018

INTENT: To construct an urgent care animal hospital within two phases. The first phase would be 8,000 s.f. of first floor hospital space with 3,000 s.f. of second floor office. The second phase would be an additional 4,800 s.f. of hospital space attached to the first floor. A related CUP application for wetland buffer impact will also appear before the Planning Board).

UNITS PROPOSED: N/A

AGENDA ITEM #: 1

ACREAGE: 5.77

ZONING DISTRICT: B-4 and Wetland Overlay

EXISTING LAND USE: Vacant

PROPOSED LAND USE: Urgent Care Animal Hospital

SURROUNDING LAND USE: Landscaping Company and Residential

ZBA ACTION: Z09-11: Variance granted on 10/15/2009 to allow a coffee shop with a drive-thru service

PERMITS REQUIRED: CUP for Wetland Protection District

WAIVERS REQUESTED: None

ATTENDANCE:

Members:

Donna Benton, Planning
Elena Piekut, Zoning
Jim Maxfield, Inspection Services
Frank Torr, Planning Board
Alan Dews, Engineering
Dan Barufaldi, Economic Development
Sgt. Speidel, Police

Others:

Chris Parker, Planning
Steve Haight, Engineer
Wes Chapin, Animal Hospital
Bob Meader, Budel Construction Corp.

PLAN REVIEW:

Planning:

- Water/Sewer investment fees
- Impact fees (\$0.92 x 11,000 s.f. = \$10,120)
- Is the plan to construct all parking in Phase I? What is the timing for Phase II?
- Remove note 17 on sheet 5 regarding hours for audible system
- Provide:
 - Landscape Plan (submitted at TRC)
 - Landscape Operation and Maintenance Plan
 - Streetscape
- Revise plan set to:
 - Add common site notes 35, 36 & 37
 - Add Planning File # P18-57
 - Add owner's signatures
 - Add PE, LLA, Licensed Landscape Architect signatures and stamps
 - Correct note 16 on cover sheet to Building "Official"
 - Show sewer easement on site plan and confirm it won't conflict with front yard/parking/access around building and into site
 - Show existing fence on existing conditions plan
 - Revise note 4 on existing conditions sheet and note 3 on sheet 5 that front setback is 30' and to remove the note about 150' from residential
 - Switch orientation on sheet 4
 - Comply with visual buffer
 - Clear up/label setback lines on sheets 4 & 5
 - Provide detail of R6-1 sign proposed
 - Pylon sign must be low planter or monument type
 - Indicate materials on Elevations and show full side (fix cut off)
 - Add permit numbers
 - Indicate zoning district boundaries



CITY OF DOVER

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Location: 67 Knox Marsh Road (Tax Map H, Lot 33)
Date: November 29, 2018

- Revise lighting plan to make darkest spot in parking lot between 0.3 and 0.5 foot candles
- Show snow storage areas
- Add wetland buffer placards
- Remove redundant notes

- Clarify the means used to maintain a clear and unobstructed accessible route from the van space to the entrance.
- Site plans shall indicate confirmation of design has been reviewed for and complies with all applicable accessibility regulations. (i.e. '09, IBC Chapter 11 w/ NH Amendments and ANSI) in accordance with NH RSA 155-A: 5.
- Provide additional building elevation plans.

Engineering Comments:

- Show the water gates at the main or tee.
- Winter maintenance should include no or low salt use.
- What is the proposed easement to 33A for?
- Per Section 149-14 A (2) this is a development causing 20,000 sf of continuous disturbance and/or 4,000 sf of new impervious are) that the applicant be asked to fill in the PTAPP tracking form.

Economic Development:

- Favor shakes not vinyl
- What will the addition look like?
- Building looks out of scale
- What is the process of surgical and medical waste disposal?

Police Department Comments:

- Clarify the special note on Sheet 4. Construct the intersection as shown on the plan, despite what the revised trip generation numbers say.
- Any potential equestrian service? Site layout is not conducive to trailer parking.
- On site or off site cremation?
- Review dumpster location in proximity to wetland buffer, consider relocating to west corner of site
- Clarify or remove note 17 on Sheet 5; presumably a holdover from the previously approved plan
- The word “alarm” is not required in note 20 on Sheet 1. Security system may be satisfied with suitable construction materials per Chapter 58, especially if business has 24 hour staff coverage. But an alarm system is still strongly encouraged, as are other security measures.

Planning Board Comments:

- Indicate materials on the exterior
- Screen dumpster

Next Steps: Come back for an unofficial TRC with letter indicating how comments were addressed and then submit for Planning Board.

TRC was paused at 11:08am.

Fire/Inspections Comments:

- Clarify location and elevation of business sign at road for clear and unobstructed view.
- Address shall be visible from street- suggest address being placed on sign.
- Discuss the entrances and exits to the building and intended use of the proposed addition.
- Indicate the use of the second floor and if staff may be sleeping within the structure.
- Clarify egress means from covered dog walking area. Provide gate if fenced area.



CITY OF DOVER

TECHNICAL REVIEW COMMITTEE MINUTES - P18-45

Application Type: Subdivision
Applicant(s): R.V. Paolini Construction LLC
Owner(s): R.V. Paolini Construction LLC
Location: 56-58 Dover Point Rd (Tax Map M, Lot 6)
Date: November 29, 2018

INTENT: To construct 6 single family detached homes and 1 garage with 1 residential unit above as well as a 400 ft long private roadway using Transfer of Development Rights.

LOTS/UNITS PROPOSED: Four new for six total dwelling units on one lot

AGENDA ITEM #: 2

ACREAGE: 1.93 acres (84,234 sq. ft.)

ZONING DISTRICT: R-40

EXISTING LAND USE: Two single family homes

SURROUNDING LAND USE: Residential and cemetery

ZBA ACTION: None

PERMITS REQUIRED: EPA Notice of Intent
NHDES Sewer Discharge

WAIVERS REQUESTED:
155-48: Roadway width

ATTENDANCE:

Members:

Donna Benton, Planning
Elena Piekut, Zoning
Jim Maxfield, Inspection Services
Sgt. Speidel, Police
Frank Torr, Planning Board
Dan Barufaldi, Economic Development
Alan Dews, Engineering

Others:

Chris Parker, Planning
Chris Berry, Engineer
Robert Paolini, Owner

PLAN REVIEW:

Planning Comments:

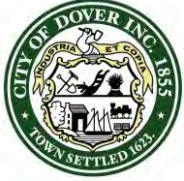
- Impact fees (7 units x \$8,895=\$62,265)
- Water/sewer investment fees
- Provide HOA documents/restrictive covenants
- Payment for transfer of development rights will be a condition of approval to be met prior to signing of plans (\$13,402 x 4=\$53,608)
- Who will maintain ownership of Centennial and of Cottage Park Drive?
- Please revise plan set to:
 - Better screen transformer to the Assistant City Manager in consultation with Eversource
 - Indicate "maximum" parking and provided parking in parking table on sheet 4
 - Include PE, LLS, Licensed Landscape Architect, and soil scientist stamp and signature on final plan set
 - Move fence so that it does not cross lot line

Police Department Comments:

- Agree with Fire about no parking/fire lane signs on both sides of street and at turnaround
- Units 4 and 1 correct bulb design to allow 2 cars in driveway

Fire/Inspections Comments:

- For clarity sheet 4 note 23 shall read Building Official not Building Inspector or remove note as a second note is located on sheet 6 note #20.
- No parking Fire lane signs should clarify no parking on either side of street.
- Electric meter locations of units 2, 4, 5, 6, and 7 do not appear to meet the location requirements of Eversource based on past practices.
- Consider re location of transformer at the end of the street. Area could be used as snow storage and possibly help with UGE routes.
- Sheet 4 note 36 should read RSA 155-A :5



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Owner(s): R.V. Paolini Construction LLC
Location: 56-58 Dover Point Rd (Tax Map M, Lot 6)
Date: November 29, 2018

- Add a note that curb boxes should be in the lawn area or if in pavement use a road box.
- Change the hydrant detail to call for eddy hydrant
- Winter maintenance should include no or low salt use.
- Per Section 149-14 A (2) this is a development causing 20,000 sf of continuous disturbance and/or 4,000 sf of new impervious are) that the applicant be asked to fill in the PTAPP tracking form.

Economic Development Comments: None

Planning Board Comments:

- Meets TDR vision

Next Steps:

Submit for the December 11 Planning Board meeting by noon on December 3, 2018

TRC paused at 11:33am.

Engineering Comments:

- Due to the lack of need to turn right out of Cottage Dr., you could tighten up the radius so that the light pole can stay



CITY OF DOVER

TECHNICAL REVIEW COMMITTEE MINUTES – P18-58

Application Type: Site Plan Review
Applicant(s): 44 Maple Ave LLC
Owner(s): Dennis Ciotti Revocable Trust
Location: 44 Maple St (Tax Map 30, Lot 19)
Date: November 29, 2018

INTENT: Demolish existing warehouse, and construct two six unit buildings, with parking located behind the buildings, and on an adjacent lot.

UNITS PROPOSED: 12

AGENDA ITEM #: 3

ACREAGE: 0.64

ZONING DISTRICT: I-1

EXISTING LAND USE: Warehouse

PROPOSED LAND USE: Multifamily residential

SURROUNDING LAND USE: Industrial and residential (single, two, and multi)

ZBA ACTION: N/A

PERMITS REQUIRED: CUP for Wetland Protection

WAIVERS REQUESTED: Parking lot setback, site specific soils

ATTENDANCE:

Members:

Donna Benton, Planning
Elena Piekut, Zoning
Jim Maxfield, Inspection Services
Sgt. Speidel, Police
Frank Torr, Planning Board
Dan Barufaldi, Economic Development
Alan Dews, Engineering

Others:

Chris Parker, Planning
Chris Berry, Engineer
Scott O'Neill
Brian O'Neill
John O'Neill

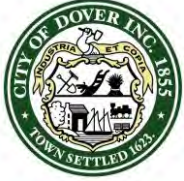
PLAN REVIEW:

Planning:

- Impact fees: \$55,212.00 (12 apartment units)
- TDR fee: \$120,618 (9 units X 13,402)
- Water/Sewer investment fees
- Consider having a pest control specialists review the site before PB
- Lauren's Drive will need a "private road" sign
- Please provide:
 - Documentation of easement over Free Trade parcel
 - Letters to serve from private utilities?
- Plan set notes/revisions:
 - Update abutters
 - Correct note 29, sheet 5 to note that there are two multi-family buildings
 - LLS, PE, CWS, and Landscape architect sign and stamp plans
 - Keep the 2 X 2 blank area for approval stamp on sheets 4 & 5
 - Add a note about solid waste disposal
 - Update file number to P18-58
 - Add Zoning District Boundaries to existing conditions plan
 - Add wetland buffer placards
 - Parking lot seems dark
 - Parking lot table on sheet 5 is different from 170-19.D(3)
 - Front setback on sheet 5 was calculated per 170-19.D.2(a)
 - PB should acknowledge first floor commercial is not being required due to use of TDR

Engineering Comments:

- Elongate the level spreader to match the edge of the parking area. This will capture all the snow loading and the contaminants in it.
- Move the watergate closer to the main
- Eliminate the hydrant
- Have a gate for each fireline and domestic line outside the building.
- Eliminate the smh and increase the service size into the buildings to 6inch. Use a wye to connect the two services.



CITY OF DOVER

TECHNICAL REVIEW COMMITTEE MINUTES – P18-58

Application Type: Site Plan Review
Applicant(s): 44 Maple Ave LLC
Owner(s): Dennis Ciotti Revocable Trust
Location: 44 Maple St (Tax Map 30, Lot 19)
Date: November 29, 2018

- OHU and drop pole—to be reviewed. May need to be underground from pole.
- Winter maintenance should include no or low salt use.
- Per Section 149-14 A (2) this is a development causing 20,000 sf of continuous disturbance and/or 4,000 sf of new impervious area that the applicant be asked to fill in the PTAPP tracking form.
- Retaining wall detail appears to include a construction safety detail. Clarify if there will be permanent provisions for fall protection.
- Indicate nearest hydrant

Economic Development:

- Update floor plan with washer and dryer
- Supportive of attractive building and good use of space

Police Department Comments:

- Unless a street name is required by code, remove it (you'll have to change "Lauren" anyway). Street addresses should be tied to 44 Maple.
- Check the "Halfinger Drive" note on Sheet 5
- Crosswalks not necessary for these site driveways, and will not be maintained by City (Remove)
- Condos or apartments? This will be important relative to the disabled space.
- Which sidewalks will be radiant?
- Hydrant location doesn't seem conducive to firefighting.
- Sheet 11: why is a free standing light provided in rear lot but not south lot?
- Sheets 13, 14: sight distance graphs are nice but do not account for horizontal curves or other road features besides grade

Planning Board Comments:

- Snow storage on easement plan (sheet 5)
- Note de-icing needs regarding Berry Brook
- Consider guard rail atop wall on parking lot
- Remove the word "optional" from vinyl
- Curb side pick note

Next Steps: Come back for an unofficial second TRC before submitting for Planning Board.

Adjournment: A motion to adjourn TRC was made at 12:25 pm by F.Torr and seconded by D.Barufaldi. Vote: U/A

Fire/Inspections Comments:

- Clarify the means used to maintain a clear and unobstructed accessible route from the van space to the entrance.
- Second parking lot may need a van accessible parking space.
- Sheet 5 note 36 should read NH RSA 155-A:5.
- Clarify access into utility room.
- Indicate the location of the FDC.
- Plan set shall indicate the installation of an approved backflow preventer for suppression system.
- Sheet 5 note 22 shall indicate the Building Official assigns addresses.
- Confirm compliance with Chapter 97 regarding solid waste



December 6, 2018

Kirt Schuman, Planning Board Chairperson
City of Dover Planning Board
288 Central Avenue
Dover, NH 03820

Attn: Christopher Parker, AICP
Assistant City Manager: Director of Planning and Strategic Initiatives

**RE: Technical Review Committee Cover
Site Plan – Lilac Garden Trash Compactor Site – 1 Lilac Lane – Tax Map H Lot 35-D-0
MSC Project #47123.00**

Dear Mr. Schuman:

On behalf of the applicant, Lilac Garden, LLC, enclosed please find a Technical Review Committee (TRC) submission relative to the above-referenced project. The following materials are included in this submission:

- Application for Conditional Use Permit (including Abutter List, Estimated Fee Schedule, & Site Plan Review Submission Checklist);
- Conditional Use Permit Request Letter
- Ground & Aerial Photographs of the Site;
- List of Professional Agents to Be Notified for Planning Board; and
- Drainage Analysis.
- Site Plans entitled “Site Development Plans, Tax Map H Lot 35-D-0, Lilac Garden Trash Compactor Site, 1 Lilac Lane, Dover, New Hampshire”, prepared by MSC a division of TFMoran, Inc., dated December 6, 2018 (7 sets at 22”x34” and 1 set at 11”x17”);

The project includes the development of a trash compactor area to support waste removal for the 50-acre existing housing complex called Lilac Garden. The proposed site is located within a Wetland Protection District Overlay District and Secondary Groundwater Protection Overlay District. The applicant seeks approval of a Conditional Use Permit for work within the Wetland Projection Overlay District.

The proposed project includes less than 4,000 sf of impervious area of which only 235 sf will be within the 50’ Wetland Buffer. Total land disturbance within the 50’ Wetland Buffer is 5,225 sf. The project includes associated stormwater treatment system, electric utilities, lighting, landscaping, and grading, resulting in approximately a quarter acre of total disturbed area.

TFMoran, Inc.
48 Constitution Drive, Bedford, NH 03110
T(603) 472-4488 www.tfmoran.com



MSC a division of TFMoran, Inc.
170 Commerce Way–Suite 102, Portsmouth, NH 03801
T(603) 431-2222 www.tfmoran.com



Technical Review Committee Cover Letter
Site Plan – Lilac Garden Trash Compactor Site – 1 Lilac Lane - Tax Map H Lot 35-D-0
MSC Project #47123.00

December 6, 2018

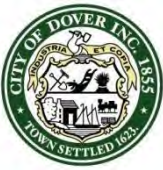
We respectfully request that we be placed on the upcoming agenda for the Technical Review Committee on December 27th. We look forward to presenting this project to you in the near future.

Respectfully,
MSC a division of TFMoran, Inc.

A handwritten signature in blue ink that reads 'Hannah Giovannucci'.

Hannah Giovannucci, PE
Project Manager

HEG



City of Dover, New Hampshire CONDITIONAL USE PERMIT APPLICATION

[Revision Date: May 18, 2017]

Office Use Only	Project #: _____	Date Received: _____
	Amount Paid: _____	Time Received: _____

APPLICANT AND OWNER INFORMATION

Name of Applicant: Lilac Garden LLC Telephone # 603-231-4440

Address of Applicant: 1 Lilac Lane, Dover, NH 03820

E-Mail Address: stephenfee34@gmail.com

Name of Property Owner (if different from applicant): _____ Telephone # _____

Address of Property Owner: _____

PROPERTY INFORMATION

Assessor's Map # H Lot(s) # 35-D-0

Address of Property: Lilac Lane, Dover, NH

Zoning District(s) RM-SU Overlay District(s) Wetland Protection & Secondary Groundwater Protection

Existing Use of Property: dwelling, multifamily

CONDITIONAL USE PERMIT INFORMATION

Type of Conditional Use Permit (Check All That Apply):

- | | | |
|---|---|--|
| ☐ Conservation District | ☐ RCM Use Overlay District | ☐ I-1 District Uses |
| ☐ Groundwater Protection | ☐ Off-Street Parking and Loading | ☐ Alternative Treatment Center |
| ☒ Wetland Protection District | ☐ Central Business District | ☐ Heritage Residential District |

Describe Proposed Use or Activity That Requires Conditional Use Permit and Describe Any Impacts:

Wetland Protection District - The applicant requests approval for the construction of approximately 235 SF of impervious area and 4,495 sf of disturbance associated with grading, access, lighting, stormwater and erosion and sediment control BMP's, utilities, and landscaping within the 50' Wetland Buffer to support the construction of a proposed trash compactor area.

List Any Associated State or Federal Permits That Have Been or Will Be Applied For and Indicate Their Status: _____

Name of Professional That Prepared Plans: Hannah Giovannucci, PE - MSC a division of TFMoran, Inc.

Address 170 Commerce Way - Suite 102, Portsmouth, NH Telephone #: 603-431-2222

Professional License #: 15699 E-mail address: hgiovannucci@tfmoran.com

SIGNATURES

I/We hereby submit this application to the City of Dover Planning Board and attest that to the best of my knowledge all of the information on this application form and in the accompanying application materials and documentation is true and accurate. As applicant or as agent, I attest that I am duly authorized to act in this capacity.

Signature of Property Owner:  Date: 11/28/2018

Signature of Applicant (if different from owner): _____ Date: _____

Signature of Agent: _____ Date: _____

AUTHORIZATION TO ENTER SUBJECT PROPERTY

I, and my successors, hereby authorize members of the Dover Planning Board, Planning Department and other pertinent City Departments and boards to enter my property for the purpose of evaluating this application, including performing inspections during the application phase, post-approval phase, construction phase and occupancy phase. It is understood that these individuals must use all reasonable care, courtesy, and diligence when on the property.

Signature of Property Owner:  Date: 11/28/2018

CITY OF DOVER CONDITIONAL USE LIST OF ABUTTERS

Pursuant to RSA 676:4, the State Law of New Hampshire, the City of Dover is required to notify the applicant, abutters (including holders of conservation easements), and any professional whose seal is on the plan, of the public hearing by certified mail, return receipt requested. Staff will provide the abutter information, while the applicant must provide accurate contact information for the owner, applicant and professional agents representing the project.

See attached Sheets

Owner:

TAX MAP	LOT #	PROPERTY OWNER	MAILING ADDRESS

Applicant (if different from owner):

APPLICANT NAME	APPLICANT COMPANY	MAILING ADDRESS

Surveyor and/or Engineer/Professional Agent:

NAME	COMPANY	MAILING ADDRESS

Conservation Easement Holder:

TAX MAP	LOT #	NAME OF EASEMENT HOLDER	MAILING ADDRESS



Civil Engineers
Structural Engineers
Traffic Engineers
Land Surveyors
Landscape Architects
Scientists

Abutters List

**Lilac Garden LLC
Knox Marsh Road / Lilac Lane
Dover, NH**

Date: **November 27, 2018**
Project #: **47123.00**

Assessors Map		Abutter Name	Mailing Address
Map	Lot		
H	36-B-1	NSA Optivest Acquisition Holdings, LLC	24901 Dana Point Harbor Drive Suite #200 Dana Point, CA 92629
H	36-B-0	Vasco Four Twenty-Seven River, LLC	217 Knox Marsh Road Dover, NH 03820-5402
H	36-C-0	Public Service Co of N.H.	P.O. Box 330 Manchester, NH 03105-0330
H	36-1-0	1 Progress Llc	340 Central Ave #202 Dover NH 03820
H	35-C-2A	Cole IO Dover NH LLC	c/o Irving Oil Marketing Inc. P.O. Box 868 Calais, ME 04619
H	35-C-1	W Coneys Properties, LLC	17 Ivans Lane Dover, NH 03820-9132
H	34-0-0	Sampson Supermarkets, Inc.	P.O. Box 1000 - MS 6000 Portland, ME 04104-5005
H	41-L-0	Crosshill Housing Associates, LP	P.O. Box 250 New Ipswich, NH 03071
H	41-D-0	Crosshill Housing Associates, LP	Same as H-41-L-0
H	41-M-0	Crosshill Housing Associates, LP	Same as H-41-L-0
H	41-R-0	MSM Brothers Inc.	510 Martha's Way Dover, NH 03820



Abutters List - cont.

Assessors Map		Abutter Name	Mailing Address
Map	Lot		
H	41-S-0	MSM Brothers Inc.	Same as H-41-R-0
H	58-0-0	City of Dover	288 Central Avenue Dover, NH 03820
H	59-0-0	Candia South Branch Brook Holdings, LLC	P.O. Box 410 Candia, NH 03034
H	60-0-0	Candia South Branch Brook Holdings, LLC	Same as H-59-0-0
H	62-0-0	Candia South Branch Brook Holdings, LLC	Same as H-59-0-0
H	63-0-0	City of Dover	Same as H-58-0-0
H	40-0-0	Olde Madbury Lane Inc.	P.O. Box 250 Dover, NH 03820
H	40-B-0	237 Albany Street Investments LLC 85% & Abel and Brunell Associates 15%	160 Middlesex Turnpike Bedford, MA 01730

Civil Engineers / Surveyor	MSC, a division of TFMoran, Inc. 170 Commerce Way – Suite 102 Portsmouth, NH 03801
Environmental / Wetlands Scientist	
Architect	

PLANNING BOARD ESTIMATED FEE SCHEDULE

This is only provided to give you an estimate of the application fees. *After you submit an application, staff will invoice you a total.* All fees must be paid two weeks before the Planning Board meeting. **If the Fee has not been paid, the item will not be placed upon the agenda.**

A. TOTAL FEE paid by cash or check made payable to "City of Dover"

1. Application fee for the following:

- SUBDIVISION \$150.00 x # _____ new lots created = \$ _____
- LOT LINE ADJUSTMENT \$200.00 (if more than two lots involved, add \$100.00 per lot) = \$ _____
- SITE REVIEW - RESIDENTIAL \$100.00 x # _____ per dwelling unit = \$ _____
- SITE REVIEW – NON-RESIDENTIAL (not to exceed \$10,000)
New construction \$.15 sq. ft. x # _____ sq. ft. = \$ _____
Additions (new floor space) \$.10 per sq. ft. x # _____ sq. ft. = \$ _____
- IMPERVIOUS PAVED AREA (for new development) OR IMPERVIOUS PARKING LOT ADDITIONS (not to exceed \$10,000) \$.07 sq. ft. x # _____ = \$ _____
- MOTEL/HOTEL \$35.00 x # _____ per lodging unit = \$ _____
- CHANGE OF USE (not to exceed \$5,000)
Existing floor space \$.10 per sq. ft. x # _____ sq. ft. = \$ _____
- CONDITIONAL USE PERMIT \$150.00 x # _____ per application = \$ _____
- GRAVEL PIT/ EXCAVATIONS - Application fee \$50.00 & Permit fee \$75.00 = \$ _____
- EXTENSIONS OF/AMENDMENTS TO/WAIVERS FOR AN APPROVED PLAN (\$150.00 minimum) \$50.00 x # _____ per hour of review \$ _____
- REQUEST FOR REZONING \$150.00 = \$ _____
- DRIVEWAY WAIVER – \$100.00 application fee = \$ _____
Letter of rejection from Engineering Department, diagram & letter from owner \$ _____

B. Abutter Notification/Mailing Labels - this office will create and print the abutter list and provide labels in triplicate for each abutter. The applicant/owner will need to provide to this office the engineer, architect, land surveyor or soil scientist whose professional seal appears on the plan with names and addresses for notices. You will be notified with the amount due, once this has been completed.

1. Applicant & Owner, engineer, architect, land surveyor or soil scientist
Certified letters fee # _____ of x \$8.00 = \$ _____
2. Certified letters fee: # of abutters _____ X \$8.00 = \$ _____
3. First Class Mail fee (for individual owner of units within a condominium or other collective form of ownership): # of abutters _____ X \$1.00 = \$ _____
4. Creating/Printing Abutter Labels in triplicate per sheet _____ x \$10.00 = \$ _____

C. Foster's newspaper public notice fee **\$ 80.00**

D. Digital Version of the Plan in PDF/A format or Archive Fee \$1.00 x # _____
per sheet of plan set = \$ _____

(Revised May 15, 2017)

TOTAL FEE \$ _____



City of Dover, New Hampshire SITE PLAN REVIEW SUBMISSION CHECKLIST

[Revision Date: May 15, 2017]

This site plan review checklist, as required by Chapter 149-6-B(2), should be completed by the applicant. It is intended to assist the applicant in the planning process of preparing a Site Plan Review application for Planning Board action. The size of the project will determine the types of information required for review, therefore, a **pre-application conference** with the Planning Department to determine the list of items that must be completed is strongly encouraged.

Fees will be invoiced and are due along with fifteen (15), folded, copies of the plan set and materials, including response to TRC comments, will be due 3 weeks prior to the Planning Board meeting.

The applicant is cautioned that this checklist is only a guide and is not intended to be a complete list of all site plan review requirements. Please refer to the Site Plan Review Regulations for full details.

APPLICANT: Lilac Lane LLC File Number: _____

PROJECT TITLE: Lilac Garden Trash Compactor Site

PROPERTY LOCATION: Lilac Lane Tax Map: H Lot: 35-D-0

	Explain How Provided	Reviewed
1. Completed and signed Application form		
2. Electronic copy of the engineered/surveyed plan layout		
3. Electronic copy of supplementary materials and application		
4. Traffic Impact Assessment and Analysis (Standard or Advanced)		
5. Fiscal Impact Analysis (For more than 10 units)		
6. Waiver requests to the Site Plan Review Regulations, with written justification		
7. Conditional Use Permit applications		
8. A colored rendering of the streetscape that will be created along the existing public right-of-way		
9. A colored architectural plan showing all sides of buildings		
10. List of all professional agents to be notified for Planning Board		
11. Eight copies of site plan w/scale of not less than 1"=50' or 1"=100' for larger site plans, for TRC. Plans shall contain the following items as appropriate:		
Location map at appropriate scale		
Proposed Project name and title and Planning File #		
Date, north arrow and scale		
Names of all abutting property owners		
Name and address of owners and/or applicants		
Signature & stamp of NH licensed land surveyor and/or engineer		
Zoning District boundaries, including any special or overlay districts		
Location of Conservation District areas		
Location, names and widths of existing and proposed streets, including pavement widths, grades, curbs and crosswalks		
Location and widths of existing & proposed easements & right of ways		
Depict pedestrian walkways and accessible access		
Location and width of existing and proposed access/egress ways		
Existing and proposed property lines with dimensions and bearings tied into Dover's Geographic Information System coordinate system		
Existing and proposed topographic information at two foot intervals		

	Explain How Provided	Reviewed
Existing and proposed buildings and structure locations		
Minimum building setbacks or build to lines on all lots		
Location and size of existing and proposed electric, telephone, gas cable and other underground utilities		
Existing and proposed water lines and fire hydrants, including materials and capacity needed		
Location and materials of sanitary sewage facilities within project site and projected additional peak hour sewer load		
Existing and proposed stormwater lines and facilities		
Lighting plan depicting all existing and proposed exterior light fixtures and a lighting level analysis for parking lots		
Location of Flood Hazard Zone		
Location of all bodies of water and watercourses		
Location of wetlands and buffers		
Neighborhood plan with aerial underlay showing how project relates to abutting uses		
Existing natural features and/or significant vegetation on property		
Depict existing contours up to 100 ft. beyond project limits		
Landscape plan depicting existing and proposed landscaping, prepared by a licensed Professional Landscape Architect		
Location of parking layout delineating spaces and arrangement; note addressing maximum required spaces		
Location, material and size of existing and proposed pavement area		
Location of proposed fire lanes		
Specify finished floor elevations of buildings		
Location of solid waste disposal facilities (dumpster, pad and screening, etc.), including required screening		
Depict all service, storage, loading bays and utility areas		
All applicable Dover Common Site Plan notes		
12. Additional Information if appropriate		
Stormwater Management Plan depicting the existing & proposed storm drainage system and engineered drainage analysis		
Stormwater Management System Operation and Maintenance Plan		
Erosion and Sedimentation Control Plan		
Landscape Operation and Maintenance Plan		
Proposed restrictive covenants or homeowners association documents		
Dates and permit numbers of all required state and federal permits		
If wetland buffer, indicate placards locations, as applicable		
Depict test boring locations, groundwater elevations and soil profiles and/or soils types call-outs		
Location of proposed drive-in facilities		
Provide additional exhibits/technical data determined appropriate by the Planning Board or its staff as required		

REVIEWED BY: _____ DATE _____ TRC DATE _____



December 6, 2018

Kirt Schuman, Planning Board Chairperson
City of Dover Planning Board
288 Central Avenue
Dover, NH 03820

Attn: Christopher Parker, AICP
Assistant City Manager: Director of Planning and Strategic Initiatives

**RE: Condition Use Permit Request
Site Plan – Lilac Garden Trash Compactor Site – 1 Lilac Lane – Tax Map H Lot 35-D-0
MSC Project #47123.00**

Dear Mr. Schuman:

On behalf of the applicant, Lilac Garden, LLC, we respectfully request a Condition Use Permit for work within the Wetlands Protection Overlay District as part of the submittal for the above-referenced project. Based on our interpretation of the City of Dover Zoning Code Chapter 170 27.1, we are requesting approval for construction within the Wetland Buffer, including, but not limited to, paved access, utilities and lighting, stormwater management systems, landscaping, and grading.

The project includes the development of a trash compactor area to support waste removal for the 50-acre existing housing complex called Lilac Garden. The proposed site is located within a Wetland Protection District Overlay District and Secondary Groundwater Protection Overlay District. The applicant seeks approval of a Conditional Use Permit for work within the Wetland Projection Overlay District.

The proposed project includes approximately 235 sf of impervious area within the 50' Wetland Buffer. Associated improvements within the 50' Wetland Buffer include a stormwater treatment system, electric utilities, lighting, landscaping, and grading, resulting in approximately 4,495 sf of disturbed area. The entire project area is approximately a quarter acre.

Wetlands Protection Overlay District

Demonstration of Need:

The proposed construction is essential to the productive use of land outside the Wetlands Protection District because approximately two thirds of the usable land in this area off of Lilac Garden is located within the 50' Wetland Buffer. Due to the narrow geometry of land between the wetlands, the development of this land outside of the Wetlands Protection District would not be

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48 Constitution Drive, Bedford, NH 03110
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accessible nor feasible if this portion of the property could not be developed. Furthermore, the proposed trash compactor site is located centrally for residential access off of Lilac Lane and proposed within an existing wooded area, which offers natural screening to Lilac Garden residences and Knox Marsh Road.

Avoidance:

Potential impacts of the project have been avoided to the maximum extent practicable. The proposed site design is the least impacting of the considered alternatives. The design seeks to minimize impervious surfaces within the 50' Wetland Buffer to the maximum extent practicable, limiting most disturbances within the Wetland Protection Overlay District to primarily grading and stormwater management.

Minimization:

Unavoidable impacts to the wetlands have been minimized by proposing limited impervious surfaces, a stormwater treatment system, and encouraging minimal use of sodium chloride-based materials for winter maintenance. The proposed impervious surface has been minimized to only what is required for access and minimizes intrusion to the 50' Wetland Buffer. The project proposed a stormwater treatment system that will improve the collection, treatment, and infiltration of stormwater over existing conditions. The stormwater treatment system also serves to protect the wetland by collecting, treating, and improving the quality of stormwater from runoff and snow/ice melt prior to entering the wetlands. The applicant has been instructed to minimize the use of sodium chloride-based materials to the minimum necessary for safety to protect the wetlands and groundwater.

Mitigation:

The project proposes to mitigate impacts to the proposed site by proposing stormwater treatment systems and through reduced sodium chloride-based usage. The City of Dover Conservation Commission will have the opportunity to review the project during the Conservation Commission Hearing.

We look forward to your review of the enclosed request. If you require additional information, please let us know.

Respectfully,
MSC a division of TFMoran, Inc.



Hannah Giovannucci, PE
Project Manager

HEG



Civil Engineers
Structural Engineers
Traffic Engineers
Land Surveyors
Landscape Architects
Scientists

Project #47123.00

Photo Exhibits for

**Lilac Garden, LLC
1 Lilac Lane
Dover, New Hampshire**

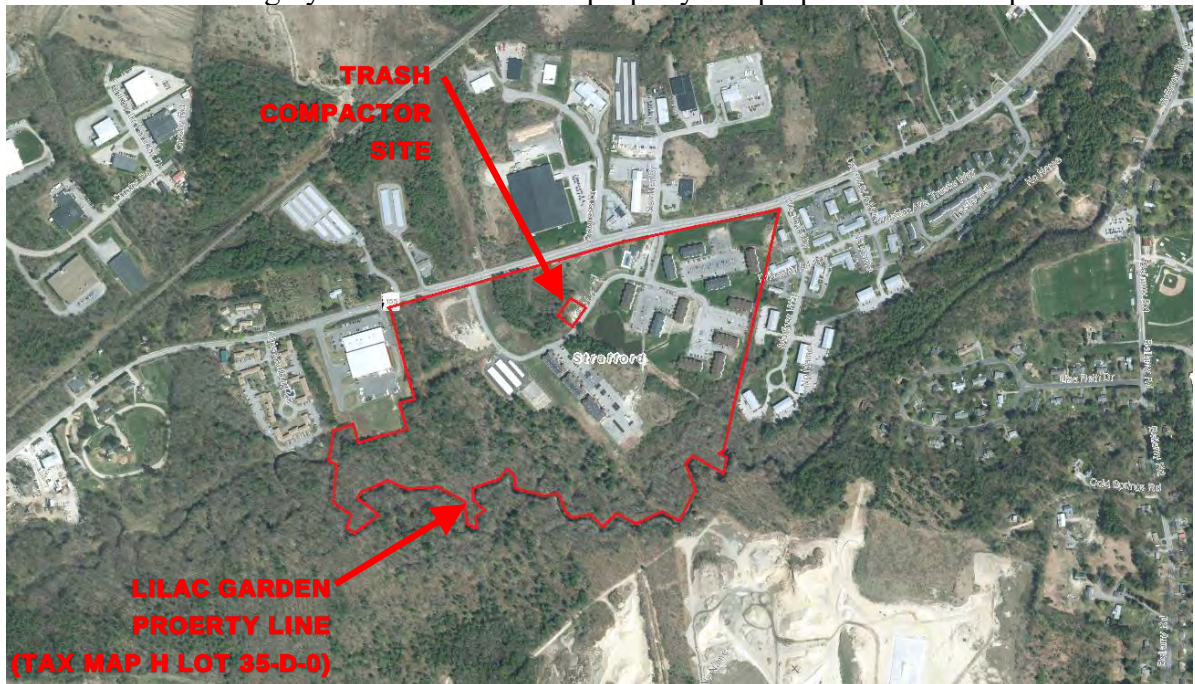
Photo 1: View from Lilac Lane of existing Lilac Garden residential trash drop-off location adjacent to the Leasing Office at the main entrance.



Photo 2: View from Lilac Lane of proposed Lilac Garden residential trash drop-off and compactor site location (see Photo 3 for Trash Compactor Site).



Photo 3: Aerial imagery of the Lilac Garden property and proposed trash compactor site.





List of Professional Agents to be Notified for Planning Board

Owner: **Lilac Garden, LLC**
C/O Stephen Fee
21 Continental Boulevard
Suite 101
Merrimack, NH 3054

Civil Engineer: **TFMoran, Inc.**
C/O Hannah Giovannucci, PE
170 Commerce Way
Suite 102
Portsmouth, NH 03801

Geotechnical Engineer: **S. W. Cole Engineering Inc.**
Nathan Cote, PE
10 Centre Road
Somersworth, NH 03878

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DRAINAGE ANALYSIS SUMMARY

F O R

Lilac Garden Compactor Site

**1 Lilac Lane
Dover, New Hampshire**

Tax Map H, Lot 35-D-0

December 6, 2018

Prepared By:



Civil Engineers
Structural Engineers
Traffic Engineers
Land Surveyors
Landscape Architects
Scientists

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1.0 SUMMARY & PROJECT DESCRIPTION

The project includes the development of a trash compactor area to support waste removal for the 50-acre existing housing complex called Lilac Garden. The lot has public access via Knox Marsh Road and internal, private access via Lilac Lane. The proposed site is located off of Lilac Lane and within a Wetland Protection District Overlay District and Secondary Groundwater Protection Overlay District. The project proposed less than 4,000 sf of impervious area for the compactor and access and ¼-acre of disturbance to facilitate the development. Associated improvements include utilities and lighting, stormwater management systems, landscaping, and grading.

The project meets and exceeds the Minimum Stormwater Protections and Management Standards required per the City of Dover Site Review Regulations Chapter 149-14(A)(2). The project also strives to meet to the maximum extent practicable the Stormwater Management for New Development, per Chapter 149-14(B), although not a requirement of this project.

This drainage study was completed to reduce the generation of stormwater runoff volume for the new development and identify the Minimum Stormwater Protections and Management Standards. The study also assesses the pre- and post-development stormwater runoff rates and volume from the proposed development for the 2-year, 10-year, 25-year, and the 50-year storm events. In addition, Low Impact Development (LID) Best Management Practices (BMP's) were designed to reduce impacts near the wetland area and protect stormwater quality both during and after construction. The following summarizes the findings from the study.

2.0 - CALCULATION METHODS

The design storms analyzed in this study are the 2-year, 10-year, 25-year and the 50-year, 24-hour storm events. The software program, HydroCAD version 10.00¹ was utilized to calculate the peak runoff rates from these storm events. The program estimates the peak rates using the TR-20 method. A Type III storm pattern was used in the model. Rainfall frequencies for the analyzed region were also incorporated into the model. Rainfall frequencies from the Northeast Regional Climate Center (Extreme Precipitation Values) were used to determine the storm-event intensities. An additional 15% of the Extreme Precipitation Values were added to each storm event for this analysis in accordance with NH AoT Regulations. See Appendix A and Table 1 below. Design standards were taken from the New Hampshire Stormwater Manual, December 2008².

¹ HydroCAD version 10.00, HydroCAD Software Solutions LLC, Chocorua, NH, 2013.

² New Hampshire Stormwater Manual: Volume One - Stormwater and Antidegradation, December 2008; Volume Two - Post-Construction Best Management Practices Selection and Design, December 2008; Volume Three - Erosion and Sediment Controls During Construction, December 2008.

Table 1 PRECIPITATION ESTIMATES	
Storm- Event (yr.)	Extreme Precipitation Rainfall Value (in.)
2	3.14
10	4.74
25	5.99
50	7.16

Time of Concentration (T_c) is the time it takes for water to flow from the most hydrologically most remote point to the watershed outlet following the longest hydrologic route within a subcatchment. This time is determined by calculating the time it takes runoff to travel this route under one of three hydrologic conditions: sheet flow, shallow concentrated flow or channel flow. Because the Intensity-Duration-Frequency (IDF) curve is steep with short T_c 's, estimating the actual intensity is subject to error and overestimates actual runoff. Due to this, the T_c 's are adjusted to a minimum of 6 minutes.

The soils within the project are identified in accordance with the NRCS soil survey (see Appendix B for soil location). The predominant soils were the 234 Biddeford Silty Clay Loam (very poorly drained) and the 233 Scantic Silt Loam (poorly drained). See Appendix B for a more detailed description and test pit logs.

3.0 – PRE-DEVELOPMENT & POST DEVELOPMENT CONDITIONS

The post-development condition is characterized by one watershed. Pre-development subcatchment areas are depicted on the attached plan entitled "Pre-Development Drainage Map," Sheet D-01 in Appendix F.

The site drains to the existing eastern wetlands which discharge via underground culvert to the other side of Lilac Lane. For ease of calculation, the storage in the existing wetlands were not evaluated.

The post-development condition is characterized by one watershed split into two subcatchment areas. Post-development subcatchment areas are depicted on the attached plan entitled "Post-Development Drainage Map," sheet D-02 in Appendix F.

A bioretention area is proposed to reduce the generation of stormwater runoff volume from the impact of the new impervious area from the proposed development. Please refer to Appendix C for the BMP Worksheet for Bioretention Area #1. The practice is lined due to shallow water tables and low soil infiltration rates.

Bioretention areas are Low Impact Development (LID's) stormwater treatment Best Management Practices (BMP's). The use of the bioretention area BMP accomplishes many of the LID goals. It is a decentralized method of treating water. The water is treated near where it is collected through vegetation and soil filtering. The plants and the root systems as well as the soil filters treat and attenuate the stormwater flows, lowering their

impact. Bioretention Area #1 is placed to act as a natural buffer between the development and wetland area.

Table 2 and Table 3 summarize the pre- and post-development peak runoff rates and volumes for the 2-year, 10-year, 25-year and 50-year 24-hour Type III storm events for POI-1. Appendices D and E in this Drainage Study documents the computations for these peak runoff rates and volumes.

TABLE 2 - PEAK RUNOFF RATE COMPARISON (CFS)					
POINT OF INTEREST		DESIGN STORM			
		2-year	10-year	25-year	50-year
POI-1	Pre	0.92	2.11	3.11	4.10
	Post	0.31	1.03	1.49	1.93

TABLE 3 – SURFACE WATER RUNOFF VOLUME COMPARISON (CF)					
POINT OF INTEREST		DESIGN STORM			
		2-year	10-year	25-year	50-year
POI-1	Pre	1,959	4,388	6,551	8,703
	Post	1,792	4,386	6,643	8,865

The proposed project reduces the generation of stormwater runoff volume from the impact of the new impervious area from the proposed development, per the required Minimum Protections and Management Standards.

The stormwater management system is further designed to reduce the pre- and post-development runoff rates and surface water runoff volumes to the maximum extent practicable, per the Stormwater Management for New Development, not applicable to this project.

4.0 – BEST MANAGEMENT PRACTICES

The proposed stormwater management system includes a Low-Impact Design for a Bioretention Area per the *New Hampshire Stormwater Manual, Volumes Two and Three, December 2008*.

BMP's also provide for temporary erosion control measures during the construction process and permanent erosion control measures after construction is complete. All soil erosion and sediment control measures shall be in accordance with regulations and principles as outlined in the *New Hampshire Stormwater Manual, Volumes Two and Three, December 2008*. The intent of the outlined measures is to minimize erosion and sedimentation during construction, stabilize and protect the site from erosion after construction is complete and mitigate any adverse impacts to stormwater

quality resulting from development. Best Management Practices for this project include:

- Temporary practices to be implemented during construction.
- Permanent practices to be implemented after construction.

4.1 – Temporary Practices:

1. Erosion, sediment, and stormwater detention measures must be installed as directed by the engineer.
2. All disturbed areas, as well as loam stockpiles, shall be seeded and contained by a silt barrier.
3. Silt barriers must be installed prior to any construction commencing. All erosion control devices including silt barriers and storm drain inlet filters shall be inspected at least once per week and following any rainfall. All necessary maintenance shall be completed within twenty-four (24) hours.
4. Any silt barriers found to be failing must be replaced immediately. Sediment is to be removed from behind the silt fence if found to be one-third the height of the silt barrier or greater.
5. Any area of the site, which has been disturbed and where construction activity will not occur for more than twenty-one (21) days, shall be temporarily stabilized by mulching and seeding.
6. No construction materials shall be buried on-site.
7. After all areas have been stabilized, temporary practices are to be removed, and the area they are removed from must be smoothed and revegetated.
8. Areas must be temporarily stabilized within 14 days of disturbance or seeded and mulched within 3 days of final stabilization.
9. After November 15th, incomplete driveways or parking areas must be protected with a minimum of 3" of crushed gravel, meeting the standards of NHDOT item 304.3.
10. An area is considered stabilized if it has met one of the following:
 - a) A minimum of 85% vegetative growth has been established.
 - b) Base course gravel has been installed in areas to be paved.
 - c) Stone, rip rap, or any other non-erosive material has been installed with a minimum thickness of 3".
 - d) Erosion control blankets have been installed.

4.2 – Permanent Practices:

The objectives for developing permanent Best Management Practices for this site include the following:

1. Mitigate existing runoff flow characteristics.
 - a) Drainage is structured to reduce or meet to the maximum extent practicable any offsite increase in runoff
2. Treatment BMP's are established to ensure the water quality.
3. Maintenance schedules are set to safeguard the long term working of the stormwater BMP's.

5.0 – REGULATORY COMPLIANCE

The project meets and exceeds the Minimum Stormwater Protections and Management Standards required per the City of Dover Site Review Regulations Chapter 149-14(A)(2).

(i) Where applicable, all DEVELOPMENT must comply with the EPA Phase II Stormwater Rules and the City's MS4 Permit as amended.

Not applicable to the proposed project.

(ii) Existing surface waters, including ponds, rivers, perennial and intermittent streams (natural or channelized), and wetland (including vernal pools) shall be protected by the minimum buffer setback as specified in Chapter 170-27. STORMWATER and EROSION and SEDIMENT control BEST MANAGEMENT PRACTICES (BMP's) shall be located outside the specified buffer zone unless otherwise approved by the Planning BOARD.

The project seeks a Conditional Use Permit within the Wetland Protection District for the construction of approximately 235 sf of impervious area and 4,495 sf of disturbance associated with grading, access, lighting, stormwater and erosion and sediment control BMP's, utilities, and landscaping within the 50' Wetland Buffer to support the construction of a proposed trash compactor area.

(iii) LOW IMPACT DEVELOPMENT (LID) site planning and design strategies shall be used to the MEP in order to reduce the generation of the STORMWATER RUNOFF volume for both new and REDEVELOPMENT projects. An owner/APPLICANT must document why LID strategies are not appropriate if not used to manage STORMWATER.

The proposed project is designed to mimic natural topography and employs a bioretention area to meet LID criteria. As summarized in Table 3, the proposed project

reduces the generation of stormwater runoff volume from the impact of the new impervious area from the proposed development.

The stormwater management system is further designed to reduce the pre- and post-development runoff rates and surface water runoff volumes to the maximum extent practicable, per the Stormwater Management for New Development, not applicable to this project.

(iv) All proposed STORMWATER practices and measures shall be installed and maintained in accordance with manufacturers' specifications and performance specifications outlined in the NH Stormwater Management Manual Volume 2 (December 2008 or current revision) a copy of which can be found on the NHDES website.

All proposed stormwater practices including Bioretention Area #1 and the Sediment Forebay have been designed accordingly. Please refer to Appendix C for the BMP Worksheet for Bioretention Area #1.

(v) All storage facilities for fuel, chemicals, chemical or industrial wastes, and biodegradable raw materials shall meet the regulations of the NHDES including but not limited to those involving underground storage tanks, above ground storage tanks, hazardous waste and BEST MANAGEMENT PRACTICES for Groundwater Protection (ENV-Wa 401).

No storage facilities will be located within the proposed project.

(vi) Salt and other de-icing materials shall be stored under cover or located such that no direct DISCHARGES to receiving waters are possible. Snow storage areas shall be located such that no direct DISCHARGES to receiving waters are possible. See NHDES guidance fact sheet on road salt and snow disposal.

Salt or other de-icing materials shall not be stored within the limits of the proposed project. Snow and ice shall be stored in area that do not directly discharge to receiving waters and are pretreated by Bioretention Area #1.

(vii) The use of sodium chloride-based materials for winter maintenance shall be the minimum necessary for safety. The use of sodium chloride-based materials is discouraged within the groundwater protection districts.

The use of salt or other de-icing materials shall be the minimum necessary for safety. Alternative methods to salt are encouraged for use within the project limits which is within the Groundwater Protection District.

(viii) IMPERVIOUS surfaces for parking areas and roads shall be minimized to the extent possible.

Impervious surfaces for parking and driveway areas have been minimized for functionality and access to the site to the maximum extend practicable.

(ix) STORMWATER RUNOFF from new IMPERVIOUS surfaces shall be directed to a BEST MANAGEMENT PRACTICE for flow MITIGATION, treatment and/or INFILTRATION.

All stormwater runoff from proposed impervious surfaces are designed to be captured by Bioretention Area #1 for flow mitigation and treatment. As summarized in Table 2, the stormwater management system is designed to reduce the pre- and post-development runoff rates.

6.0 – CONCLUSION

The proposed project meets and exceeds the Minimum Protections and Management Standards as required by the Site Development Criteria per the City of Dover Site Review Regulations. The proposed design and stormwater management system mitigates the proposed impervious area and disturbances associated with the project. The proposed bioretention areas will treat the impervious cover and control the stormwater runoff rates and volumes. Appropriate erosion and sediment control practices will be implemented to reduce possible erosion and siltation. Best Management Practices will be developed in accordance with the *New Hampshire Stormwater Manual, Volumes Two and Three, December 2008* to formulate a plan that assures stormwater quality both during and after construction.

Submitted by,
TFMoran, Inc.

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APPENDIX A – EXTREME PRECIPITATION **TABLE**

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Extreme Precipitation Tables

Northeast Regional Climate Center

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

Smoothing	Yes
State	New Hampshire
Location	
Longitude	70.903 degrees West
Latitude	43.178 degrees North
Elevation	0 feet
Date/Time	Mon, 05 Nov 2018 17:28:32 -0500

Extreme Precipitation Estimates

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.26	0.40	0.50	0.65	0.81	1.03	1yr	0.70	0.98	1.20	1.54	1.99	2.60	2.85	1yr	2.30	2.74	3.14	3.87	4.44	1yr
2yr	0.32	0.49	0.61	0.81	1.01	1.28	2yr	0.87	1.17	1.50	1.91	2.44	3.14	3.49	2yr	2.78	3.35	3.85	4.58	5.21	2yr
5yr	0.37	0.57	0.72	0.96	1.23	1.58	5yr	1.06	1.44	1.85	2.38	3.07	3.97	4.47	5yr	3.51	4.29	4.91	5.79	6.54	5yr
10yr	0.40	0.63	0.80	1.09	1.42	1.84	10yr	1.22	1.69	2.18	2.82	3.65	4.74	5.39	10yr	4.19	5.18	5.91	6.91	7.77	10yr
25yr	0.46	0.74	0.94	1.29	1.72	2.26	25yr	1.48	2.09	2.69	3.52	4.60	5.99	6.91	25yr	5.30	6.65	7.56	8.74	9.77	25yr
50yr	0.51	0.83	1.06	1.48	1.99	2.66	50yr	1.72	2.45	3.17	4.17	5.48	7.16	8.35	50yr	6.34	8.03	9.10	10.44	11.62	50yr
100yr	0.58	0.93	1.20	1.70	2.31	3.11	100yr	2.00	2.88	3.73	4.94	6.52	8.57	10.09	100yr	7.58	9.70	10.97	12.49	13.83	100yr
200yr	0.64	1.04	1.35	1.94	2.69	3.66	200yr	2.32	3.39	4.41	5.88	7.78	10.25	12.19	200yr	9.07	11.72	13.22	14.94	16.46	200yr
500yr	0.75	1.24	1.61	2.34	3.28	4.52	500yr	2.83	4.21	5.47	7.35	9.80	13.00	15.67	500yr	11.50	15.07	16.92	18.93	20.75	500yr

Lower Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.24	0.37	0.45	0.60	0.74	0.90	1yr	0.64	0.88	0.91	1.25	1.56	2.06	2.48	1yr	1.83	2.38	2.91	3.34	4.04	1yr
2yr	0.31	0.49	0.60	0.81	1.00	1.18	2yr	0.86	1.16	1.36	1.83	2.36	3.04	3.39	2yr	2.69	3.26	3.74	4.46	5.05	2yr
5yr	0.35	0.54	0.67	0.91	1.16	1.40	5yr	1.00	1.37	1.61	2.15	2.78	3.70	4.12	5yr	3.28	3.96	4.59	5.40	6.11	5yr
10yr	0.38	0.59	0.73	1.02	1.32	1.60	10yr	1.14	1.57	1.82	2.45	3.13	4.27	4.78	10yr	3.78	4.60	5.31	6.24	7.01	10yr
25yr	0.44	0.66	0.83	1.18	1.55	1.91	25yr	1.34	1.87	2.11	2.85	3.66	5.01	5.79	25yr	4.44	5.57	6.48	7.56	8.43	25yr
50yr	0.48	0.73	0.91	1.31	1.76	2.19	50yr	1.52	2.14	2.35	3.20	4.12	5.74	6.69	50yr	5.08	6.44	7.53	8.74	9.68	50yr
100yr	0.54	0.81	1.02	1.47	2.02	2.51	100yr	1.74	2.45	2.63	3.59	4.60	6.56	7.72	100yr	5.81	7.43	8.76	10.09	11.09	100yr
200yr	0.60	0.90	1.14	1.65	2.30	2.87	200yr	1.98	2.81	2.93	4.02	5.15	7.50	8.92	200yr	6.64	8.57	10.18	11.66	12.73	200yr
500yr	0.70	1.04	1.33	1.94	2.76	3.46	500yr	2.38	3.38	3.40	4.66	5.99	8.91	10.77	500yr	7.89	10.35	12.42	14.14	15.21	500yr

Upper Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.28	0.43	0.53	0.71	0.87	1.08	1yr	0.75	1.05	1.23	1.74	2.21	2.84	3.06	1yr	2.51	2.94	3.39	4.19	4.77	1yr
2yr	0.33	0.51	0.62	0.84	1.04	1.25	2yr	0.90	1.22	1.48	1.95	2.50	3.26	3.60	2yr	2.88	3.46	3.97	4.72	5.40	2yr
5yr	0.39	0.60	0.75	1.03	1.31	1.58	5yr	1.13	1.55	1.85	2.49	3.19	4.22	4.81	5yr	3.74	4.63	5.24	6.19	6.95	5yr
10yr	0.46	0.70	0.87	1.21	1.57	1.92	10yr	1.35	1.87	2.23	3.04	3.84	5.20	6.00	10yr	4.60	5.77	6.58	7.59	8.47	10yr
25yr	0.55	0.84	1.05	1.49	1.97	2.47	25yr	1.70	2.41	2.87	3.95	4.93	6.97	8.04	25yr	6.17	7.74	8.77	9.99	11.04	25yr
50yr	0.64	0.97	1.21	1.74	2.34	2.98	50yr	2.02	2.92	3.48	4.81	5.98	8.61	10.07	50yr	7.62	9.68	10.92	12.28	13.50	50yr
100yr	0.74	1.12	1.41	2.03	2.78	3.60	100yr	2.40	3.52	4.22	5.89	7.27	10.63	12.60	100yr	9.41	12.11	13.60	15.12	16.51	100yr
200yr	0.86	1.29	1.64	2.37	3.31	4.36	200yr	2.85	4.27	5.13	7.20	8.81	13.17	15.79	200yr	11.65	15.18	16.94	18.60	20.23	200yr
500yr	1.05	1.56	2.01	2.92	4.15	5.60	500yr	3.58	5.48	6.63	9.42	11.40	17.50	21.27	500yr	15.49	20.45	22.66	24.50	26.48	500yr

APPENDIX B – SITE SOILS REPORT & TEST PIT LOGS

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United States
Department of
Agriculture

NRCS

Natural
Resources
Conservation
Service

A product of the National
Cooperative Soil Survey,
a joint effort of the United
States Department of
Agriculture and other
Federal agencies, State
agencies including the
Agricultural Experiment
Stations, and local
participants

Custom Soil Resource Report for **Strafford County, New Hampshire**



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (<http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/>) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (<https://offices.sc.egov.usda.gov/locator/app?agency=nrcs>) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

Custom Soil Resource Report

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Custom Soil Resource Report Soil Map



MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

Special Point Features

 Blowout

 Borrow Pit

 Clay Spot

 Closed Depression

 Gravel Pit

 Gravelly Spot

 Landfill

 Lava Flow

 Marsh or swamp

 Mine or Quarry

 Miscellaneous Water

 Perennial Water

 Rock Outcrop

 Saline Spot

 Sandy Spot

 Severely Eroded Spot

 Sinkhole

 Slide or Slip

 Sodic Spot

 Spoil Area

 Stony Spot

 Very Stony Spot

 Wet Spot

 Other

 Special Line Features

Water Features

 Streams and Canals

Transportation

 Rails

 Interstate Highways

 US Routes

 Major Roads

 Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Strafford County, New Hampshire
Survey Area Data: Version 18, Sep 5, 2018

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Dec 31, 2009—Jun 26, 2016

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol		Map Unit Name	Acres in AOI	Percent of AOI
Be	234	Biddeford silty clay loam	0.4	21.8%
ScA	233	Scantic silt loam, 0 to 3 percent slopes	1.3	78.2%
Totals for Area of Interest			1.7	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however,

onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Strafford County, New Hampshire

Be—Biddeford silty clay loam

Map Unit Setting

National map unit symbol: 9d6m
Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F
Frost-free period: 140 to 240 days
Farmland classification: Not prime farmland

Map Unit Composition

Biddeford and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Biddeford

Setting

Landform: Marine terraces
Parent material: Glaciomarine

Typical profile

H1 - 0 to 7 inches: silty clay loam
H2 - 7 to 26 inches: silty clay
H3 - 26 to 41 inches: silty clay

Properties and qualities

Slope: 0 to 3 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Very poorly drained
Runoff class: High
Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately high (0.00 to 0.20 in/hr)
Depth to water table: About 0 inches
Frequency of flooding: None
Frequency of ponding: Frequent
Available water storage in profile: Moderate (about 6.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 6w
Hydrologic Soil Group: C/D
Hydric soil rating: Yes

Minor Components

Not named wet

Percent of map unit: 10 percent
Landform: Marine terraces
Hydric soil rating: Yes

Scantic

Percent of map unit: 5 percent
Landform: Marine terraces
Hydric soil rating: Yes

ScA—Scantic silt loam, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: 9d8s
Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F
Frost-free period: 140 to 240 days
Farmland classification: Farmland of local importance

Map Unit Composition

Scantic and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Scantic

Setting

Landform: Marine terraces

Typical profile

H1 - 0 to 13 inches: silt loam
H2 - 13 to 23 inches: silty clay loam
H3 - 23 to 40 inches: silty clay

Properties and qualities

Slope: 0 to 3 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Poorly drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately high (0.00 to 0.20 in/hr)
Depth to water table: About 0 to 12 inches
Frequency of flooding: None
Frequency of ponding: None
Available water storage in profile: Moderate (about 6.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 4w
Hydrologic Soil Group: C/D
Hydric soil rating: Yes

Minor Components

Not named wet

Percent of map unit: 5 percent
Landform: Marine terraces
Hydric soil rating: Yes

Swanton

Percent of map unit: 5 percent

Custom Soil Resource Report

Landform: Marine terraces

Hydric soil rating: Yes

Biddeford

Percent of map unit: 5 percent

Landform: Marine terraces

Hydric soil rating: Yes

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Custom Soil Resource Report

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Soil Field Data Sheet

Soil Profile: TP-1

Date, Time, Weather: 11/28/2018, 9:30am, Cloudy.

Top Elevation: 91 ft ±

Base Elevation: 86.2 ft ±

Horizon	Depth	Texture	Rock Fragments	Structure	Consistence	Permeability	Matrix Color	Color Redox Features	pH	Roots	Notes
N/A	0"-18"	silt loam	∅	blocky	friable	slow	10YR 6/4	∅	∅	common v. f. - fine	FILL
N/A	18"-58"	si-y cl loam	∅	platy	firm	slow	10YR 7/4	common medium	∅	few very fine	FILL w/ free water at 58".

Soil Formation Process: Alluvium Re-Worked (FILL)

Land Position and Parent Material: Floodplains

Vegatative Cover: Grass Field

Redoximorphic Features Depth: Relic and New at Elevation 89.5 ft ±

High Seasonal Water Table Depth: Elevation 89.5 ft ±

Restrictive Layers: Silty Clay Loam at Elevation 89.5 ft ±

Drainage Class and Land Use Potential: Building Site Development

Notes: Elevations estimated from "Test Pit Location Plan" prepared by MSC and dated November 15, 2018.

Soil Field Data Sheet

Soil Profile: TP-2

Date, Time, Weather: 11/28/2018, 9:08am, Cloudy.

Top Elevation: 91 ft ±

Base Elevation: 85.9 ft ±

Horizon	Depth	Texture	Rock Fragments	Structure	Consistence	Permeability	Matrix Color	Color Redox Features	pH	Roots	Notes
N/A	0"-18"	silt loam	∅	blocky	friable	slow	10YR 6/4	∅	∅	common v. f. - fine	FILL
N/A	18"-58"	si-y cl loam	∅	blocky	firm	slow	10YR 7/4	common medium	∅	few very fine	FILL w/ free water at 61".

Soil Formation Process: Alluvium Re-Worked (FILL)

Land Position and Parent Material: Floodplains

Vegatative Cover: Grass Field

Redoximorphic Features Depth: Relic and New at Elevation 89.5 ft ±

High Seasonal Water Table Depth: Elevation 89.5 ft ±

Restrictive Layers: Silty Clay Loam at Elevation 89.5 ft ±

Drainage Class and Land Use Potential: Building Site Development

Notes: Elevations estimated from "Test Pit Location Plan" prepared by MSC and dated November 15, 2018.

Soil Field Data Sheet

Soil Profile: TP-3

Date, Time, Weather: 11/28/2018, 8:04am, Partly Cloudy.

Top Elevation: 91 ft ±

Base Elevation: 86.1 ft ±

Horizon	Depth	Texture	Rock Fragments	Structure	Consistence	Permeability	Matrix Color	Color Redox Features	pH	Roots	Notes
N/A	0"-9"	silt loam	∅	blocky	friable	slow	10YR 4/2	∅	∅	common v. f. - fine	FILL
N/A	9"-42"	silt loam	∅	blocky	friable	slow	10YR 6/4	∅	∅	few/com v. f. - f.	FILL
Ob	42"-59"	si-y cl loam	∅	platy	firm	slow	10YR 3/2	common m. to coarse	∅	∅	Free water at 61".

Soil Formation Process: Alluvium Re-Worked (FILL)

Land Position and Parent Material: Floodplains

Vegatative Cover: Grass Field

Redoximorphic Features Depth: Relic and New at Elevation 87.5 ft ±

High Seasonal Water Table Depth: Elevation 87.5 ft ±

Restrictive Layers: Silty Clay Loam at Elevation 87.5 ft ±

Drainage Class and Land Use Potential: Building Site Development

Notes: Elevations estimated from "Test Pit Location Plan" prepared by MSC and dated November 15, 2018.



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TEST PIT	DEPTH
TP-1	8 FT
TP-2	8 FT
TP-3	8 FT
TP-4	6 FT

NOTE:

1. TEST PITS DEPTHS LISTED ARE MINIMUM DEPTHS. IF SEASONAL HIGH WATER IS ENCOUNTERED, THE TEST PITS CAN BE STOPPED.
2. COORDINATE WITH ENGINEER ON-SITE TO PROVIDE INFILTRATION TESTING AT ALL TEST PIT LOCATIONS PER ENV-WQ 1500.14(d) AND AT LEAST 1 FOOT ABOVE SEASONAL HIGH WATER, UNLESS OTHERWISE DIRECTED.

SITE DEVELOPMENT PLANS

TAX MAP H LOT 35-D-0
TEST PIT LOCATION PLAN
LILAC GARDEN TRASH COMPACTOR SITE
LILAC LANE
DOVER, NEW HAMPSHIRE

OWNED BY
LILAC GARDEN, LLC

SCALE: 1" = 20' (22X34)
1" = 40' (11X17)

NOVEMBER 15, 2018

- Civil Engineers
- Structural Engineers
- Traffic Engineers
- Land Surveyors
- Landscape Architects
- Scientists

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F r e	47123.00	DR	RCK	FB	
		CK	JPL	CADFILE	47123.00_Test Pits.dwg



TP-1



TP-2



TP-3



OVERALL SITE

APPENDIX C – BMP WORKSHEET

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FILTRATION PRACTICE DESIGN CRITERIA (Env-Wq 1508.07)

Type/Node Name: _____

Bioretention #1

Enter the type of filtration practice (e.g., bioretention system) and the node name in the drainage analysis, if applicable

		Have you reviewed the restrictions on unlined systems outlined in Env-Wq 1508.07(a)?	
0.24	ac	A = Area draining to the practice	
0.07	ac	A _I = Impervious area draining to the practice	
0.31	decimal	I = percent impervious area draining to the practice, in decimal form	
0.33	unitless	R _v = Runoff coefficient = 0.05 + (0.9 x I)	
0.08	ac-in	WQV = 1" x R _v x A	
286	cf	WQV conversion (ac-in x 43,560 sf/ac x 1ft/12")	
71	cf	25% x WQV (check calc for sediment forebay volume)	
214	cf	75% x WQV (check calc for surface sand filter volume)	
		Method of Pretreatment? (not required for clean or roof runoff)	
382	cf	V _{SED} = sediment forebay volume, if used for pretreatment	← ≥ 25%WQV
627	sf	A _{SA} = surface area of the practice	
-	iph	K _{sat} _{DESIGN} = design infiltration rate ¹	
Yes	Yes/No	If K _{sat} (prior to factor of safety) is < 0.50 iph, has an underdrain been provided?	
-	hours	T _{DRAIN} = drain time = V / (A _{SA} * I _{DESIGN})	← ≤ 72-hrs
86.25	feet	E _{FC} = elevation of the bottom of the filter course material ²	
85.58	feet	E _{UD} = invert elevation of the underdrain (UD), if applicable	
87.50	feet	E _{SHWT} = elevation of SHWT (if none found, enter the lowest elevation of the test pit)	
86.10	feet	E _{ROCK} = elevation of bedrock (if none found, enter the lowest elevation of the test pit)	
0.67	feet	D _{FC to UD} = depth to UD from the bottom of the filter course	← ≥ 1'
0.15	feet	D _{FC to ROCK} = depth to bedrock from the bottom of the filter course	← ≥ 1'
(1.25)	feet	D _{FC to SHWT} = depth to SHWT from the bottom of the filter course	← ≥ 1'
89.73	ft	Peak elevation of the 50-year storm event (infiltration can be used in analysis)	
90.15	ft	Elevation of the top of the practice	
YES		50 peak elevation ≤ Elevation of the top of the practice	← yes

If a surface sand filter or underground sand filter is proposed:

YES	ac	Drainage Area check.	← < 10 ac
	cf	V = volume of storage ³ (attach a stage-storage table)	← ≥ 75%WQV
	inches	D _{FC} = filter course thickness	← 18", or 24" if within GPA
Sheet		Note what sheet in the plan set contains the filter course specification	
Yes/No		Access grate provided?	← yes

If a bioretention area is proposed:

YES	ac	Drainage Area no larger than 5 ac?	← yes
1,733	cf	V = volume of storage ³ (attach a stage-storage table)	← ≥ WQV
18.0	inches	D _{FC} = filter course thickness	← 18", or 24" if within GPA
Sheet	C-10	Note what sheet in the plan set contains the filter course specification	
2.0	:1	Pond side slopes	← ≥3:1
Sheet	C-06	Note what sheet in the plan set contains the planting plans and surface cover	

If porous pavement is proposed:

	Type of pavement proposed (concrete? Asphalt? Pavers? Etc)	
acres	A_{SA} = surface area of the pervious pavement	
#DIV/0! :1	ratio of the contributing area to the pervious surface area	← 5:1
inches	D_{FC} = filter course thickness	← 12", or 18" if within GPA
Sheet	Note what sheet in the plan set contains the filter course spec.	← 304.1 sand

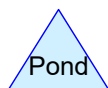
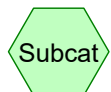
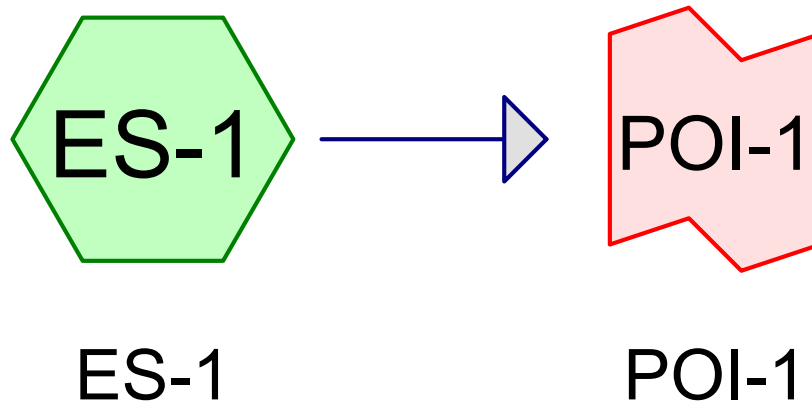
1. Rate of the limiting layer (either the filter course or the underlying soil). K_{sat_design} includes factor of safety. See Env-Wq 1504.14 for guidance on determining the infiltration rate.
2. See lines 34, 40 and 48 for required depths of filter media.
3. Volume without depending on infiltration. The volume includes the storage above the filter (but below the invert of the outlet structure, if any), the filter media voids, and the pretreatment area. The storage above the filter media shall not include the volume above the outlet structure, if any.

Designer's Notes:

[illegible]

APPENDIX D – PRE DRAINAGE

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Area Listing (all nodes)

Area (sq-ft)	CN	Description (subcatchment-numbers)
14,694	74	>75% Grass cover, Good, HSG C (ES-1)
11,641	70	Woods, Good, HSG C (ES-1)
26,335	72	TOTAL AREA

Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
0	HSG A	
0	HSG B	
26,335	HSG C	ES-1
0	HSG D	
0	Other	
26,335		TOTAL AREA

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Type II 24-hr 2 Year Rainfall=3.14"

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Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment ES-1: ES-1

Runoff Area=26,335 sf 0.00% Impervious Runoff Depth=0.89"

Flow Length=106' Slope=0.0280 '/' Tc=19.9 min CN=72 Runoff=0.55 cfs 1,959 cf

Link POI-1: POI-1

Inflow=0.55 cfs 1,959 cf

Primary=0.55 cfs 1,959 cf

Total Runoff Area = 26,335 sf Runoff Volume = 1,959 cf Average Runoff Depth = 0.89"
100.00% Pervious = 26,335 sf 0.00% Impervious = 0 sf

Summary for Subcatchment ES-1: ES-1

Runoff = 0.55 cfs @ 12.15 hrs, Volume= 1,959 cf, Depth= 0.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 2 Year Rainfall=3.14"

Area (sf)	CN	Description
14,694	74	>75% Grass cover, Good, HSG C
11,641	70	Woods, Good, HSG C
26,335	72	Weighted Average
26,335		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.9	106	0.0280	0.09		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.14"

Summary for Link POI-1: POI-1

Inflow Area = 26,335 sf, 0.00% Impervious, Inflow Depth = 0.89" for 2 Year event
Inflow = 0.55 cfs @ 12.15 hrs, Volume= 1,959 cf
Primary = 0.55 cfs @ 12.15 hrs, Volume= 1,959 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

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Type II 24-hr 10 Year Rainfall=4.74"

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Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment ES-1: ES-1

Runoff Area=26,335 sf 0.00% Impervious Runoff Depth=2.00"

Flow Length=106' Slope=0.0280 '/' Tc=19.9 min CN=72 Runoff=1.32 cfs 4,388 cf

Link POI-1: POI-1

Inflow=1.32 cfs 4,388 cf

Primary=1.32 cfs 4,388 cf

Total Runoff Area = 26,335 sf Runoff Volume = 4,388 cf Average Runoff Depth = 2.00"
100.00% Pervious = 26,335 sf 0.00% Impervious = 0 sf

Summary for Subcatchment ES-1: ES-1

Runoff = 1.32 cfs @ 12.13 hrs, Volume= 4,388 cf, Depth= 2.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 10 Year Rainfall=4.74"

Area (sf)	CN	Description
14,694	74	>75% Grass cover, Good, HSG C
11,641	70	Woods, Good, HSG C
26,335	72	Weighted Average
26,335		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.9	106	0.0280	0.09		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.14"

Summary for Link POI-1: POI-1

Inflow Area = 26,335 sf, 0.00% Impervious, Inflow Depth = 2.00" for 10 Year event
Inflow = 1.32 cfs @ 12.13 hrs, Volume= 4,388 cf
Primary = 1.32 cfs @ 12.13 hrs, Volume= 4,388 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

47123.00_PRE*Type II 24-hr 25 Year Rainfall=5.99"*

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Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment ES-1: ES-1

Runoff Area=26,335 sf 0.00% Impervious Runoff Depth=2.99"

Flow Length=106' Slope=0.0280 '/' Tc=19.9 min CN=72 Runoff=2.00 cfs 6,551 cf

Link POI-1: POI-1

Inflow=2.00 cfs 6,551 cf

Primary=2.00 cfs 6,551 cf

Total Runoff Area = 26,335 sf Runoff Volume = 6,551 cf Average Runoff Depth = 2.99"
100.00% Pervious = 26,335 sf 0.00% Impervious = 0 sf

Summary for Subcatchment ES-1: ES-1

Runoff = 2.00 cfs @ 12.13 hrs, Volume= 6,551 cf, Depth= 2.99"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 25 Year Rainfall=5.99"

Area (sf)	CN	Description
14,694	74	>75% Grass cover, Good, HSG C
11,641	70	Woods, Good, HSG C
26,335	72	Weighted Average
26,335		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.9	106	0.0280	0.09		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.14"

Summary for Link POI-1: POI-1

Inflow Area = 26,335 sf, 0.00% Impervious, Inflow Depth = 2.99" for 25 Year event

Inflow = 2.00 cfs @ 12.13 hrs, Volume= 6,551 cf

Primary = 2.00 cfs @ 12.13 hrs, Volume= 6,551 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

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Type II 24-hr 50 Year Rainfall=7.16"

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Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN

Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment ES-1: ES-1

Runoff Area=26,335 sf 0.00% Impervious Runoff Depth=3.97"

Flow Length=106' Slope=0.0280 '/' Tc=19.9 min CN=72 Runoff=2.66 cfs 8,703 cf

Link POI-1: POI-1

Inflow=2.66 cfs 8,703 cf

Primary=2.66 cfs 8,703 cf

Total Runoff Area = 26,335 sf Runoff Volume = 8,703 cf Average Runoff Depth = 3.97"
100.00% Pervious = 26,335 sf 0.00% Impervious = 0 sf

Summary for Subcatchment ES-1: ES-1

Runoff = 2.66 cfs @ 12.13 hrs, Volume= 8,703 cf, Depth= 3.97"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 50 Year Rainfall=7.16"

Area (sf)	CN	Description
14,694	74	>75% Grass cover, Good, HSG C
11,641	70	Woods, Good, HSG C
26,335	72	Weighted Average
26,335		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.9	106	0.0280	0.09		Sheet Flow, Woods Woods: Light underbrush n= 0.400 P2= 3.14"

Summary for Link POI-1: POI-1

Inflow Area = 26,335 sf, 0.00% Impervious, Inflow Depth = 3.97" for 50 Year event

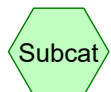
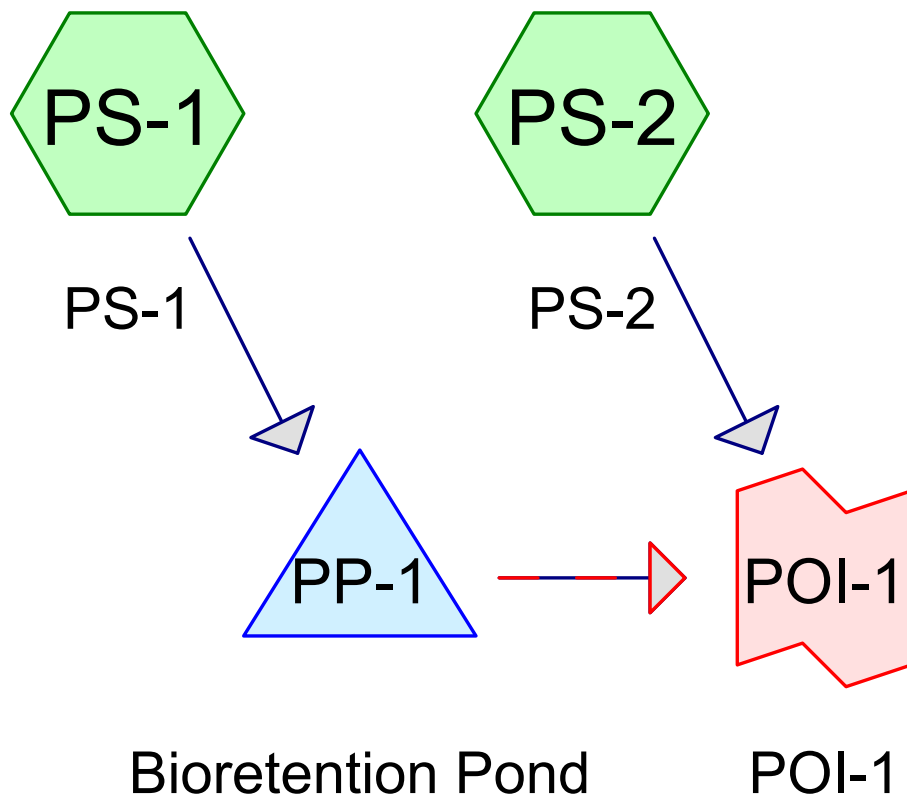
Inflow = 2.66 cfs @ 12.13 hrs, Volume= 8,703 cf

Primary = 2.66 cfs @ 12.13 hrs, Volume= 8,703 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

APPENDIX E – POST DRAINAGE

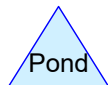
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Subcat



Reach



Pond



Link

Routing Diagram for 47123.00_POST
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Area Listing (selected nodes)

Area (sq-ft)	CN	Description (subcatchment-numbers)
11,804	74	>75% Grass cover, Good, HSG C (PS-1, PS-2)
3,230	98	Paved parking, HSG C (PS-1)
11,301	70	Woods, Good, HSG C (PS-2)
26,335	75	TOTAL AREA

Soil Listing (selected nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
0	HSG A	
0	HSG B	
26,335	HSG C	PS-1, PS-2
0	HSG D	
0	Other	
26,335		TOTAL AREA

47123.00_POST*Type II 24-hr 2 Year Rainfall=3.14"*

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Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment PS-1: PS-1

Runoff Area=10,557 sf 30.60% Impervious Runoff Depth=1.42"
Flow Length=100' Tc=10.6 min CN=81 Runoff=0.51 cfs 1,251 cf

Subcatchment PS-2: PS-2

Runoff Area=15,778 sf 0.00% Impervious Runoff Depth=0.84"
Flow Length=106' Slope=0.0280 '/' Tc=19.9 min CN=71 Runoff=0.31 cfs 1,107 cf

Pond PP-1: Bioretention Pond

Peak Elev=88.15' Storage=658 cf Inflow=0.51 cfs 1,251 cf
Primary=0.05 cfs 685 cf Secondary=0.00 cfs 0 cf Outflow=0.05 cfs 685 cf

Link POI-1: POI-1

Inflow=0.31 cfs 1,792 cf
Primary=0.31 cfs 1,792 cf

Total Runoff Area = 26,335 sf Runoff Volume = 2,358 cf Average Runoff Depth = 1.07"
87.73% Pervious = 23,105 sf 12.27% Impervious = 3,230 sf

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Type II 24-hr 2 Year Rainfall=3.14"

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Summary for Subcatchment PS-1: PS-1

Runoff = 0.51 cfs @ 12.03 hrs, Volume= 1,251 cf, Depth= 1.42"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 2 Year Rainfall=3.14"

Area (sf)	CN	Description
7,327	74	>75% Grass cover, Good, HSG C
3,230	98	Paved parking, HSG C
10,557	81	Weighted Average
7,327		69.40% Pervious Area
3,230		30.60% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.1	67	0.0220	0.11		Sheet Flow, Grass
					Grass: Dense n= 0.240 P2= 3.14"
0.5	33	0.0200	1.09		Sheet Flow, Driveway
					Smooth surfaces n= 0.011 P2= 3.14"
10.6	100	Total			

Summary for Subcatchment PS-2: PS-2

Runoff = 0.31 cfs @ 12.15 hrs, Volume= 1,107 cf, Depth= 0.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 2 Year Rainfall=3.14"

Area (sf)	CN	Description
4,477	74	>75% Grass cover, Good, HSG C
11,301	70	Woods, Good, HSG C
15,778	71	Weighted Average
15,778		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.9	106	0.0280	0.09		Sheet Flow, Woods
					Woods: Light underbrush n= 0.400 P2= 3.14"

Summary for Pond PP-1: Bioretention Pond

Inflow Area = 10,557 sf, 30.60% Impervious, Inflow Depth = 1.42" for 2 Year event
 Inflow = 0.51 cfs @ 12.03 hrs, Volume= 1,251 cf
 Outflow = 0.05 cfs @ 12.68 hrs, Volume= 685 cf, Atten= 91%, Lag= 39.2 min
 Primary = 0.05 cfs @ 12.68 hrs, Volume= 685 cf
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

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Type II 24-hr 2 Year Rainfall=3.14"

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Peak Elev= 88.15' @ 12.68 hrs Surf.Area= 656 sf Storage= 658 cf

Flood Elev= 90.50' Surf.Area= 1,088 sf Storage= 2,391 cf

Plug-Flow detention time= 284.1 min calculated for 685 cf (55% of inflow)

Center-of-Mass det. time= 161.2 min (1,002.7 - 841.5)

Volume	Invert	Avail.Storage	Storage Description	
#1	85.00'	2,391 cf	Custom Stage Data (Prismatic) Listed below (Recalc)	
Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
85.00	627	0.0	0	0
85.25	627	60.0	94	94
86.25	627	40.0	251	345
87.75	627	20.0	188	533
88.00	627	20.0	31	564
89.00	827	100.0	727	1,291
90.00	1,052	100.0	940	2,231
90.15	1,088	100.0	161	2,391

Device	Routing	Invert	Outlet Devices
#1	Device 3	85.58'	4.0" Vert. Orifice/Grate C= 0.600
#2	Device 3	89.50'	8.0" Horiz. Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	88.00'	4.0" Round Culvert L= 61.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 88.00' / 87.35' S= 0.0107 ' / ' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.09 sf
#4	Secondary	89.90'	10.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=0.05 cfs @ 12.68 hrs HW=88.15' TW=0.00' (Dynamic Tailwater)↑ **3=Culvert** (Inlet Controls 0.05 cfs @ 1.30 fps)↑ **1=Orifice/Grate** (Passes 0.05 cfs of 0.16 cfs potential flow)↑ **2=Grate** (Controls 0.00 cfs)**Secondary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=85.00' TW=0.00' (Dynamic Tailwater)↑ **4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)**Summary for Link POI-1: POI-1**

Inflow Area = 26,335 sf, 12.27% Impervious, Inflow Depth = 0.82" for 2 Year event

Inflow = 0.31 cfs @ 12.15 hrs, Volume= 1,792 cf

Primary = 0.31 cfs @ 12.15 hrs, Volume= 1,792 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

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Type II 24-hr 10 Year Rainfall=4.74"

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Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment PS-1: PS-1

Runoff Area=10,557 sf 30.60% Impervious Runoff Depth=2.76"
Flow Length=100' Tc=10.6 min CN=81 Runoff=0.98 cfs 2,425 cf

Subcatchment PS-2: PS-2

Runoff Area=15,778 sf 0.00% Impervious Runoff Depth=1.92"
Flow Length=106' Slope=0.0280 '/' Tc=19.9 min CN=71 Runoff=0.76 cfs 2,527 cf

Pond PP-1: Bioretention Pond

Peak Elev=88.70' Storage=1,054 cf Inflow=0.98 cfs 2,425 cf
Primary=0.28 cfs 1,859 cf Secondary=0.00 cfs 0 cf Outflow=0.28 cfs 1,859 cf

Link POI-1: POI-1

Inflow=1.03 cfs 4,386 cf
Primary=1.03 cfs 4,386 cf

Total Runoff Area = 26,335 sf Runoff Volume = 4,952 cf Average Runoff Depth = 2.26"
87.73% Pervious = 23,105 sf 12.27% Impervious = 3,230 sf

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Type II 24-hr 10 Year Rainfall=4.74"

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Summary for Subcatchment PS-1: PS-1

Runoff = 0.98 cfs @ 12.02 hrs, Volume= 2,425 cf, Depth= 2.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 10 Year Rainfall=4.74"

Area (sf)	CN	Description
7,327	74	>75% Grass cover, Good, HSG C
3,230	98	Paved parking, HSG C
10,557	81	Weighted Average
7,327		69.40% Pervious Area
3,230		30.60% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.1	67	0.0220	0.11		Sheet Flow, Grass
					Grass: Dense n= 0.240 P2= 3.14"
0.5	33	0.0200	1.09		Sheet Flow, Driveway
					Smooth surfaces n= 0.011 P2= 3.14"
10.6	100	Total			

Summary for Subcatchment PS-2: PS-2

Runoff = 0.76 cfs @ 12.13 hrs, Volume= 2,527 cf, Depth= 1.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 10 Year Rainfall=4.74"

Area (sf)	CN	Description
4,477	74	>75% Grass cover, Good, HSG C
11,301	70	Woods, Good, HSG C
15,778	71	Weighted Average
15,778		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.9	106	0.0280	0.09		Sheet Flow, Woods
					Woods: Light underbrush n= 0.400 P2= 3.14"

Summary for Pond PP-1: Bioretention Pond

Inflow Area = 10,557 sf, 30.60% Impervious, Inflow Depth = 2.76" for 10 Year event
 Inflow = 0.98 cfs @ 12.02 hrs, Volume= 2,425 cf
 Outflow = 0.28 cfs @ 12.23 hrs, Volume= 1,859 cf, Atten= 72%, Lag= 12.3 min
 Primary = 0.28 cfs @ 12.23 hrs, Volume= 1,859 cf
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

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Type II 24-hr 10 Year Rainfall=4.74"

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Peak Elev= 88.70' @ 12.23 hrs Surf.Area= 768 sf Storage= 1,054 cf

Flood Elev= 90.50' Surf.Area= 1,088 sf Storage= 2,391 cf

Plug-Flow detention time= 168.1 min calculated for 1,859 cf (77% of inflow)

Center-of-Mass det. time= 77.8 min (900.3 - 822.5)

Volume	Invert	Avail.Storage	Storage Description	
#1	85.00'	2,391 cf	Custom Stage Data (Prismatic) Listed below (Recalc)	
Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
85.00	627	0.0	0	0
85.25	627	60.0	94	94
86.25	627	40.0	251	345
87.75	627	20.0	188	533
88.00	627	20.0	31	564
89.00	827	100.0	727	1,291
90.00	1,052	100.0	940	2,231
90.15	1,088	100.0	161	2,391

Device	Routing	Invert	Outlet Devices
#1	Device 3	85.58'	4.0" Vert. Orifice/Grate C= 0.600
#2	Device 3	89.50'	8.0" Horiz. Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	88.00'	4.0" Round Culvert L= 61.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 88.00' / 87.35' S= 0.0107 ' / Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.09 sf
#4	Secondary	89.90'	10.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=0.28 cfs @ 12.23 hrs HW=88.70' TW=0.00' (Dynamic Tailwater)↑ **3=Culvert** (Barrel Controls 0.28 cfs @ 3.20 fps)↑ **1=Orifice/Grate** (Passes 0.28 cfs of 0.35 cfs potential flow)↑ **2=Grate** (Controls 0.00 cfs)**Secondary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=85.00' TW=0.00' (Dynamic Tailwater)↑ **4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)**Summary for Link POI-1: POI-1**

Inflow Area = 26,335 sf, 12.27% Impervious, Inflow Depth = 2.00" for 10 Year event

Inflow = 1.03 cfs @ 12.14 hrs, Volume= 4,386 cf

Primary = 1.03 cfs @ 12.14 hrs, Volume= 4,386 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

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Type II 24-hr 25 Year Rainfall=5.99"

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Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment PS-1: PS-1

Runoff Area=10,557 sf 30.60% Impervious Runoff Depth=3.87"
Flow Length=100' Tc=10.6 min CN=81 Runoff=1.37 cfs 3,409 cf

Subcatchment PS-2: PS-2

Runoff Area=15,778 sf 0.00% Impervious Runoff Depth=2.89"
Flow Length=106' Slope=0.0280 '/' Tc=19.9 min CN=71 Runoff=1.16 cfs 3,801 cf

Pond PP-1: Bioretention Pond

Peak Elev=89.25' Storage=1,508 cf Inflow=1.37 cfs 3,409 cf
Primary=0.35 cfs 2,842 cf Secondary=0.00 cfs 0 cf Outflow=0.35 cfs 2,842 cf

Link POI-1: POI-1

Inflow=1.49 cfs 6,643 cf
Primary=1.49 cfs 6,643 cf

Total Runoff Area = 26,335 sf Runoff Volume = 7,209 cf Average Runoff Depth = 3.29"
87.73% Pervious = 23,105 sf 12.27% Impervious = 3,230 sf

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Type II 24-hr 25 Year Rainfall=5.99"

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Summary for Subcatchment PS-1: PS-1

Runoff = 1.37 cfs @ 12.02 hrs, Volume= 3,409 cf, Depth= 3.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 25 Year Rainfall=5.99"

Area (sf)	CN	Description
7,327	74	>75% Grass cover, Good, HSG C
3,230	98	Paved parking, HSG C
10,557	81	Weighted Average
7,327		69.40% Pervious Area
3,230		30.60% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.1	67	0.0220	0.11		Sheet Flow, Grass
					Grass: Dense n= 0.240 P2= 3.14"
0.5	33	0.0200	1.09		Sheet Flow, Driveway
					Smooth surfaces n= 0.011 P2= 3.14"
10.6	100	Total			

Summary for Subcatchment PS-2: PS-2

Runoff = 1.16 cfs @ 12.13 hrs, Volume= 3,801 cf, Depth= 2.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 25 Year Rainfall=5.99"

Area (sf)	CN	Description
4,477	74	>75% Grass cover, Good, HSG C
11,301	70	Woods, Good, HSG C
15,778	71	Weighted Average
15,778		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.9	106	0.0280	0.09		Sheet Flow, Woods
					Woods: Light underbrush n= 0.400 P2= 3.14"

Summary for Pond PP-1: Bioretention Pond

Inflow Area = 10,557 sf, 30.60% Impervious, Inflow Depth = 3.87" for 25 Year event
 Inflow = 1.37 cfs @ 12.02 hrs, Volume= 3,409 cf
 Outflow = 0.35 cfs @ 12.25 hrs, Volume= 2,842 cf, Atten= 75%, Lag= 13.6 min
 Primary = 0.35 cfs @ 12.25 hrs, Volume= 2,842 cf
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

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Type II 24-hr 25 Year Rainfall=5.99"

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Peak Elev= 89.25' @ 12.25 hrs Surf.Area= 884 sf Storage= 1,508 cf

Flood Elev= 90.50' Surf.Area= 1,088 sf Storage= 2,391 cf

Plug-Flow detention time= 140.9 min calculated for 2,839 cf (83% of inflow)

Center-of-Mass det. time= 69.0 min (881.9 - 812.8)

Volume	Invert	Avail.Storage	Storage Description	
#1	85.00'	2,391 cf	Custom Stage Data (Prismatic) Listed below (Recalc)	
Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
85.00	627	0.0	0	0
85.25	627	60.0	94	94
86.25	627	40.0	251	345
87.75	627	20.0	188	533
88.00	627	20.0	31	564
89.00	827	100.0	727	1,291
90.00	1,052	100.0	940	2,231
90.15	1,088	100.0	161	2,391

Device	Routing	Invert	Outlet Devices
#1	Device 3	85.58'	4.0" Vert. Orifice/Grate C= 0.600
#2	Device 3	89.50'	8.0" Horiz. Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	88.00'	4.0" Round Culvert L= 61.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 88.00' / 87.35' S= 0.0107 ' ' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.09 sf
#4	Secondary	89.90'	10.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=0.35 cfs @ 12.25 hrs HW=89.25' TW=0.00' (Dynamic Tailwater)↑ **3=Culvert** (Barrel Controls 0.35 cfs @ 3.97 fps)↑ **1=Orifice/Grate** (Passes 0.35 cfs of 0.47 cfs potential flow)↑ **2=Grate** (Controls 0.00 cfs)**Secondary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=85.00' TW=0.00' (Dynamic Tailwater)↑ **4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)**Summary for Link POI-1: POI-1**

Inflow Area = 26,335 sf, 12.27% Impervious, Inflow Depth = 3.03" for 25 Year event

Inflow = 1.49 cfs @ 12.13 hrs, Volume= 6,643 cf

Primary = 1.49 cfs @ 12.13 hrs, Volume= 6,643 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

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Type II 24-hr 50 Year Rainfall=7.16"

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Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment PS-1: PS-1

Runoff Area=10,557 sf 30.60% Impervious Runoff Depth=4.95"
Flow Length=100' Tc=10.6 min CN=81 Runoff=1.73 cfs 4,358 cf

Subcatchment PS-2: PS-2

Runoff Area=15,778 sf 0.00% Impervious Runoff Depth=3.86"
Flow Length=106' Slope=0.0280 '/' Tc=19.9 min CN=71 Runoff=1.55 cfs 5,073 cf

Pond PP-1: Bioretention Pond

Peak Elev=89.73' Storage=1,957 cf Inflow=1.73 cfs 4,358 cf
Primary=0.40 cfs 3,792 cf Secondary=0.00 cfs 0 cf Outflow=0.40 cfs 3,792 cf

Link POI-1: POI-1

Inflow=1.93 cfs 8,865 cf
Primary=1.93 cfs 8,865 cf

Total Runoff Area = 26,335 sf Runoff Volume = 9,432 cf Average Runoff Depth = 4.30"
87.73% Pervious = 23,105 sf 12.27% Impervious = 3,230 sf

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Type II 24-hr 50 Year Rainfall=7.16"

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Summary for Subcatchment PS-1: PS-1

Runoff = 1.73 cfs @ 12.02 hrs, Volume= 4,358 cf, Depth= 4.95"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 50 Year Rainfall=7.16"

Area (sf)	CN	Description
7,327	74	>75% Grass cover, Good, HSG C
3,230	98	Paved parking, HSG C
10,557	81	Weighted Average
7,327		69.40% Pervious Area
3,230		30.60% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
10.1	67	0.0220	0.11		Sheet Flow, Grass
					Grass: Dense n= 0.240 P2= 3.14"
0.5	33	0.0200	1.09		Sheet Flow, Driveway
					Smooth surfaces n= 0.011 P2= 3.14"
10.6	100	Total			

Summary for Subcatchment PS-2: PS-2

Runoff = 1.55 cfs @ 12.13 hrs, Volume= 5,073 cf, Depth= 3.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 50 Year Rainfall=7.16"

Area (sf)	CN	Description
4,477	74	>75% Grass cover, Good, HSG C
11,301	70	Woods, Good, HSG C
15,778	71	Weighted Average
15,778		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.9	106	0.0280	0.09		Sheet Flow, Woods
					Woods: Light underbrush n= 0.400 P2= 3.14"

Summary for Pond PP-1: Bioretention Pond

Inflow Area = 10,557 sf, 30.60% Impervious, Inflow Depth = 4.95" for 50 Year event
 Inflow = 1.73 cfs @ 12.02 hrs, Volume= 4,358 cf
 Outflow = 0.40 cfs @ 12.26 hrs, Volume= 3,792 cf, Atten= 77%, Lag= 14.7 min
 Primary = 0.40 cfs @ 12.26 hrs, Volume= 3,792 cf
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

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Type II 24-hr 50 Year Rainfall=7.16"

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Peak Elev= 89.73' @ 12.26 hrs Surf.Area= 992 sf Storage= 1,957 cf

Flood Elev= 90.50' Surf.Area= 1,088 sf Storage= 2,391 cf

Plug-Flow detention time= 129.9 min calculated for 3,788 cf (87% of inflow)

Center-of-Mass det. time= 69.2 min (875.0 - 805.8)

Volume	Invert	Avail.Storage	Storage Description	
#1	85.00'	2,391 cf	Custom Stage Data (Prismatic) Listed below (Recalc)	
Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
85.00	627	0.0	0	0
85.25	627	60.0	94	94
86.25	627	40.0	251	345
87.75	627	20.0	188	533
88.00	627	20.0	31	564
89.00	827	100.0	727	1,291
90.00	1,052	100.0	940	2,231
90.15	1,088	100.0	161	2,391

Device	Routing	Invert	Outlet Devices
#1	Device 3	85.58'	4.0" Vert. Orifice/Grate C= 0.600
#2	Device 3	89.50'	8.0" Horiz. Grate C= 0.600 Limited to weir flow at low heads
#3	Primary	88.00'	4.0" Round Culvert L= 61.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 88.00' / 87.35' S= 0.0107 ' / Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.09 sf
#4	Secondary	89.90'	10.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=0.40 cfs @ 12.26 hrs HW=89.73' TW=0.00' (Dynamic Tailwater)

↑ **3=Culvert** (Barrel Controls 0.40 cfs @ 4.54 fps)
 ↑ **1=Orifice/Grate** (Passes < 0.55 cfs potential flow)
 ↑ **2=Grate** (Passes < 0.75 cfs potential flow)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=85.00' TW=0.00' (Dynamic Tailwater)

↑ **4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Link POI-1: POI-1

Inflow Area = 26,335 sf, 12.27% Impervious, Inflow Depth = 4.04" for 50 Year event
 Inflow = 1.93 cfs @ 12.13 hrs, Volume= 8,865 cf
 Primary = 1.93 cfs @ 12.13 hrs, Volume= 8,865 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

APPENDIX F – PRE & POST DRAINAGE PLANS

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GENERAL INFORMATION

OWNER/APPLICANT

MAP H LOT 35-D-0
LILAC GARDEN, LLC
21 CONTINENTAL BOULEVARD, SUITE 101
MERRIMACK, NH 03054

RESOURCE LIST

PLANNING DEPARTMENT
288 CENTRAL AVENUE
DOVER, NH 03820
603-516-6008
CHRISTOPHER PARKER, PLANNING DIRECTOR

BUILDING DEPARTMENT

262 SIXTH STREET
DOVER, NH 03820
603-516-6038
JAMES MAXFIELD, BUILDING OFFICIAL

PUBLIC WORKS

271 MAST ROAD
DOVER, NH 03820
603-516-6450
JOHN STORER, COMMUNITY SERVICES DIRECTOR

POLICE DEPARTMENT

46 CHESTNUT STREET
DOVER, NH 03820
603-742-4646
WILLIAM BREAUULT, POLICE CHIEF

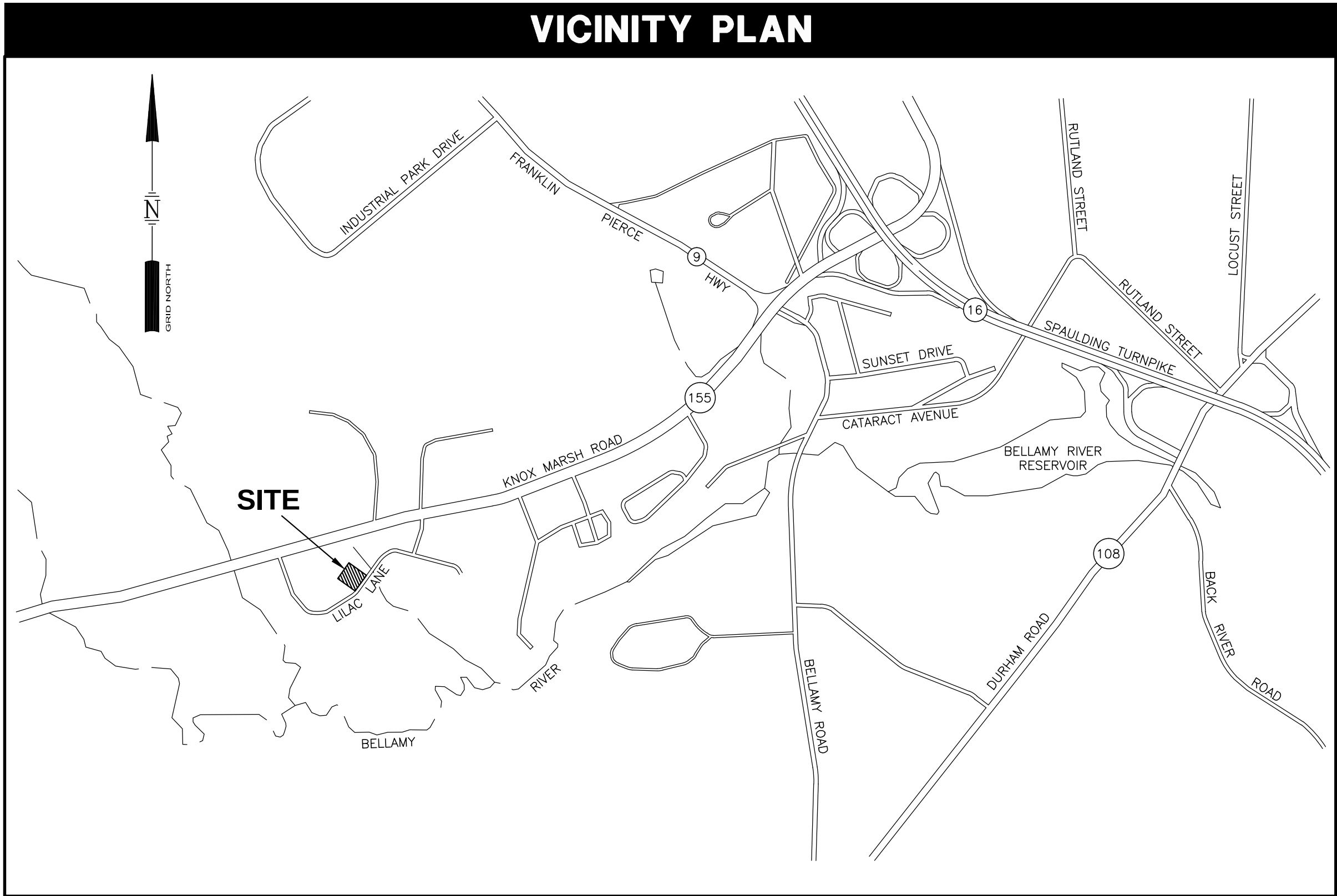
FIRE DEPARTMENT

262 SIXTH STREET
DOVER, NH 03820
603-516-6148
ERIC HAGMAN, FIRE CHIEF

LILAC GARDEN
TRASH COMPACTOR SITE

LILAC LANE
DOVER, NEW HAMPSHIRE

DECEMBER 6, 2018



Civil Engineers
Structural Engineers
Traffic Engineers
Land Surveyors
Landscape Architects
Scientists

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INDEX OF SHEETS

SHEET	SHEET TITLE
C-00	COVER
S-01	EXISTING CONDITIONS PLAN
C-01	MISCELLANEOUS NOTES
C-02	SITE & LAYOUT PLAN
C-03	GRADING, DRAINAGE, & EROSION CONTROL PLAN
C-04	UTILITY PLAN
C-05	LANDSCAPE PLAN
C-06	LANDSCAPE DETAILS
C-07	EROSION CONTROL NOTES & DETAILS
C-08 TO C-09	DETAILS
C-10	TRUCK MANEUVERING PLAN
SL1	SITE LIGHTING-LIGHTING PLAN, SCHEDULES AND SPECIFICATIONS

PERMITS/APPROVALS

NUMBER	APPROVED	EXPIRES
DOVER CONDITIONAL USE PERMIT FOR WETLAND PROTECTION DISTRICT PER ZONING CHAPTER 170-27.1(E)	-- PENDING	--

THESE PLANS ARE PERMIT DRAWINGS ONLY AND HAVE NOT BEEN DETAILED FOR CONSTRUCTION OR BIDDING.

SITE DEVELOPMENT PLANS

TAX MAP H LOT 35-D-0
COVER
LILAC GARDEN TRASH COMPACTOR SITE
1 LILAC LANE
DOVER, NEW HAMPSHIRE

OWNED BY
LILAC GARDEN, LLC

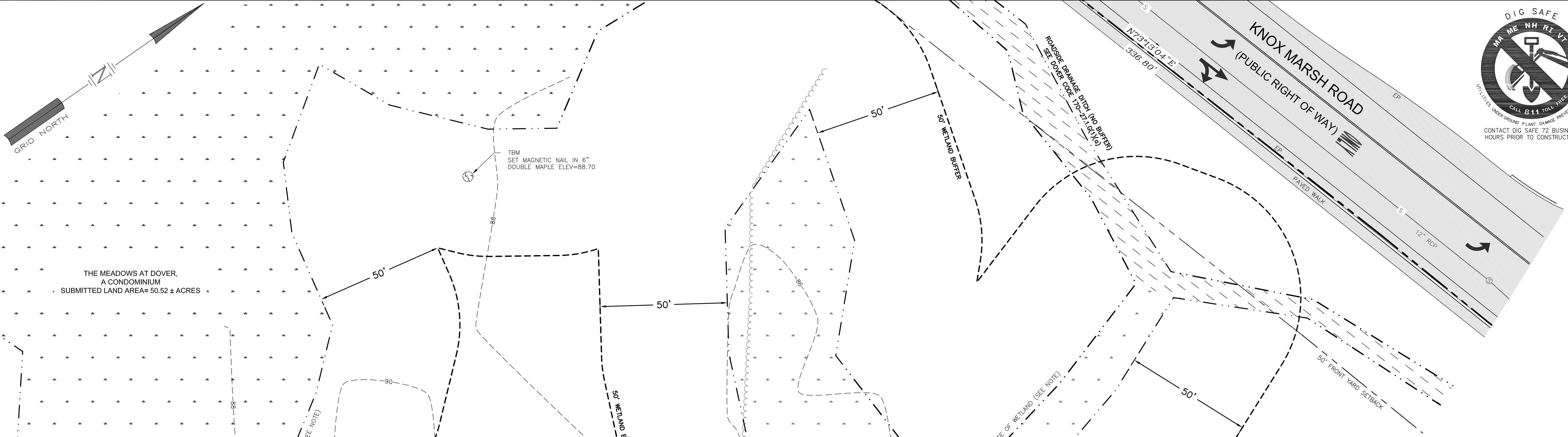
DECEMBER 6, 2018



Civil Engineers
Structural Engineers
Traffic Engineers
Land Surveyors
Landscape Architects
Scientists

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FILE	47123.00	DR	RCK	FB	47123.00_Cover.dwg	C-00
		CK	HEG	CADFILE		



CB	CATCH BASIN
ELEV	ELEVATION
EP	EDGE OF PAVEMENT
INV.	INVERT
PVC	POLYVINYL CHLORIDE PIPE
RCP	REINFORCED CONCRETE PIPE
SGC	SLOPED GRANITE CURB
SMH	SEWER MANHOLE
TBM	TEMPORARY BENCH MARK
	BOUNDARY LINE
	EXISTING CONTOUR
	EXISTING SEWER LINE
	EXISTING DRAIN LINE
	EDGE OF WOODS
	EDGE OF WETLAND
	POST AND RAIL FENCE
	LIGHT POLE
	WATER VALVE
	HYDRANT
	SEWER MANHOLE
	CATCH BASIN
	WETLANDS (WITH BUFFER)
	ROADSIDE DRAINAGE DITCH (NO BUFFER)
	RIPRAP
	PAVEMENT

SCALE: 1" = 20' (22x34)
1" = 40' (11x17) AUGUST 24, 2018

WETLAND NOTE:

THE LIMITS OF JURISDICTIONAL WETLANDS AS SHOWN ON THIS PLAN WERE DELINEATED BY GOVE ENVIRONMENTAL SERVICES, INC. ON JUNE 1, 2016, IN ACCORDANCE WITH:

1. US ARMY CORPS OF ENGINEERS' REGIONAL SUPPLEMENT TO THE CORPS OF ENGINEERS WETLAND DETERMINATION MANUAL: "NORTHWEST AND NORTHEAST REGION, TECHNICAL REPORT ERDC/EL TR-12-1, JANUARY 2012, VERSION 2.0
2. FIELD INDICATORS OF HYDRIC SOILS IN THE UNITED STATES, A GUIDE FOR IDENTIFYING AND FIELDING WETLANDS, VERSION 7.0, UNITED STATES DEPARTMENT OF AGRICULTURE (2010).
3. NORTH AMERICAN DIGITAL FLORA: NATIONAL WETLAND PLANT LIST, CURRENT VERSION.

THE LIMITS OF JURISDICTIONAL WETLANDS AS SHOWN ON THIS PLAN WERE DELINEATED BY GOVE ENVIRONMENTAL SERVICES, INC., ON JUNE 1, 2016, IN ACCORDANCE WITH:

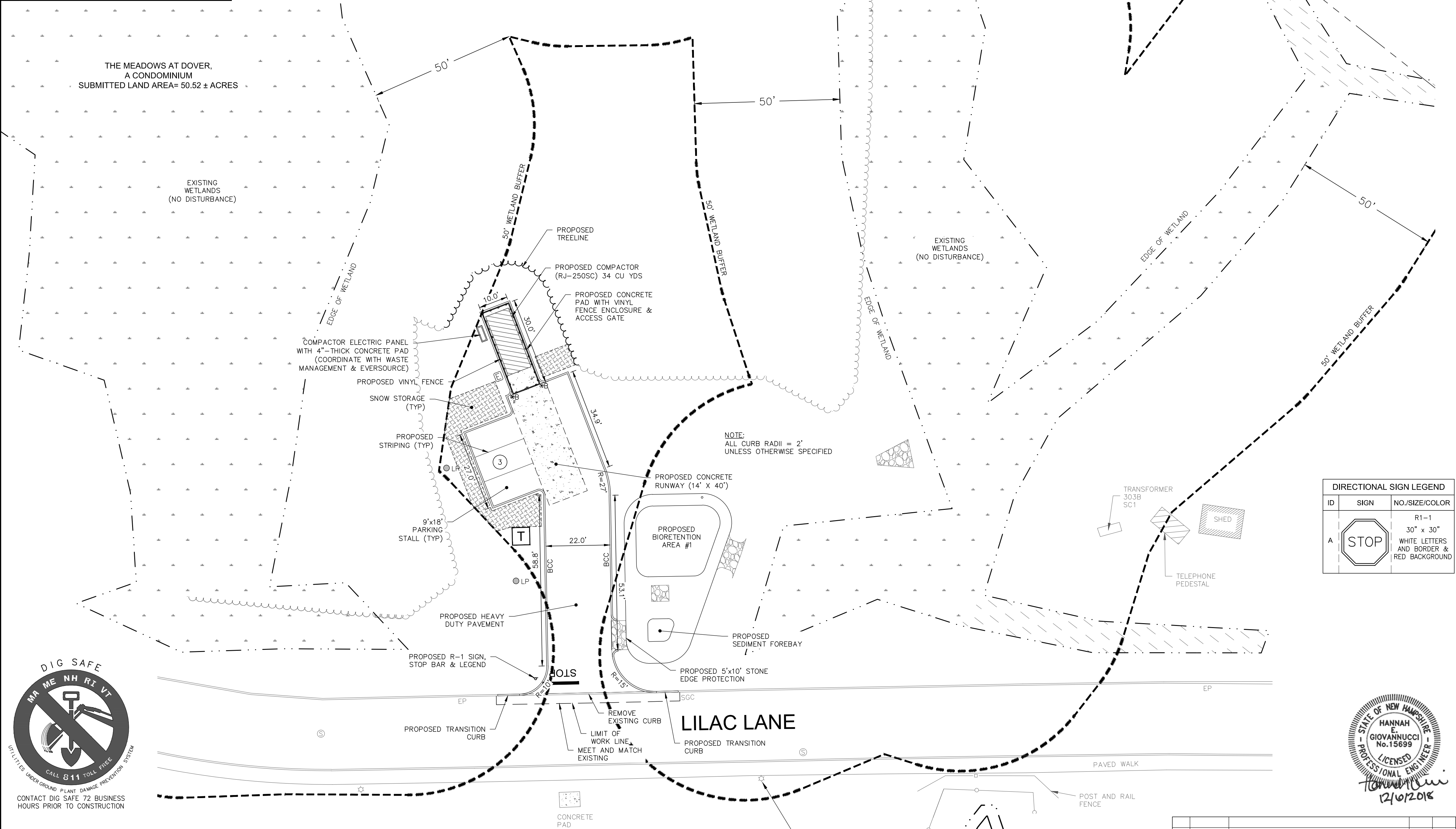
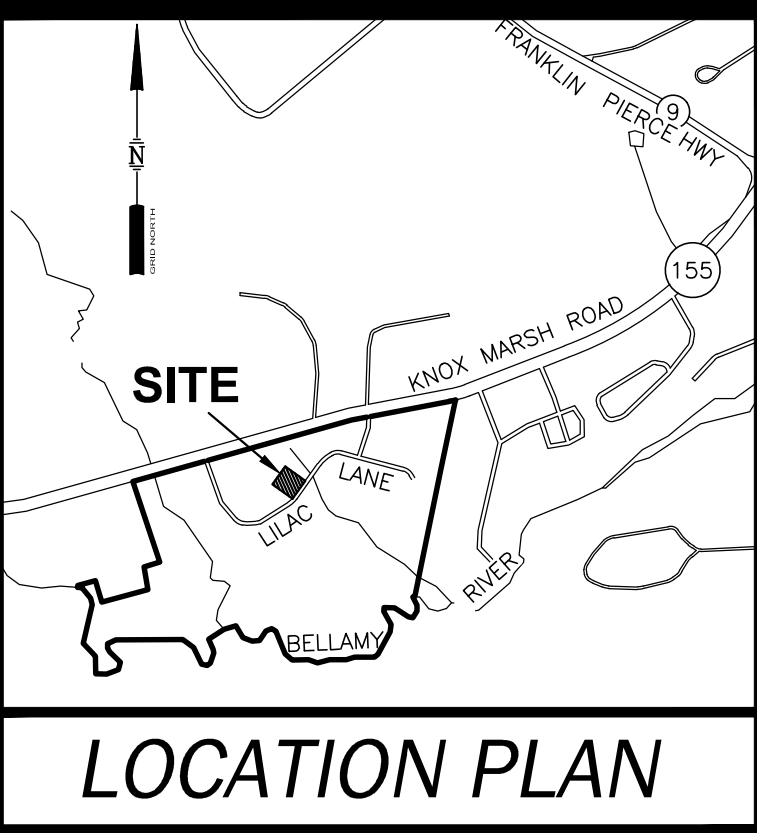
1. U.S. ARMY CORPS OF ENGINEERS REGIONAL SUPPLEMENT TO THE CORPS OF ENGINEERS WETLAND DELINEATION MANUAL: NORTH CENTRAL AND NORTHEAST REGION, TECHNICAL REPORT ERDC/EL TR-11-1, JANUARY 2011.
2. FIELD INDICATORS OF HYDRIC SOILS IN THE UNITED STATES, A GUIDE FOR IDENTIFYING AND DELINEATING HYDRIC SOILS, VERSION 7.0, UNITED STATES DEPARTMENT OF AGRICULTURE (2010).
3. NORTH AMERICAN DIGITAL FLORA: NATIONAL WETLAND PLANT LIST, CURRENT VERSION.

A graphic scale bar divided into four equal segments. Above the segments are the numbers 20, 10, 0, and 20. The first segment is black, the second is white, the third is black, and the fourth is white. Below the bar is the text "Graphic Scale in Feet".



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Dec 06, 2018 - 11:11am
F:\MSC Projects\47123 - Knox Marsh Road - Monument Garden\TRASH COMPACTOR\Design\Production Drawings\47123_00_Site.dwg



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NOTES

1. SNOW AND ICE SHALL BE STORED AS INDICATED ON THE PLAN.
2. THE USE OF SODIUM CHLORIDE-BASED MATERIALS FOR WINTER MAINTENANCE SHALL BE THE MINIMUM NECESSARY FOR SAFETY. ALTERNATIVE METHODS FOR WINTER MAINTENANCE ARE ENCOURAGED WITHIN THE TRASH COMPACTOR SITE WHICH IS WITHIN THE GROUNDWATER PROTECTION DISTRICT.

SITE DATA

ZONED USE: SUBURBAN DENSITY MULTI RESIDENTIAL (RM-SU) DISTRICT
EXISTING USE: DWELLING, MULTIFAMILY
PROPOSED USE: DWELLING, MULTIFAMILY

THE PURPOSE OF THIS PLAN IS TO CONSTRUCT A TRASH COMPACTOR SITE FOR RESIDENT'S USE ONLY

DIMENSIONAL REQUIREMENTS (CURRENT ZONING)

	REQUIRED:	PROVIDED:
MINIMUM LOT DIMENSIONS:		
LOT AREA	20,000 SF	2,200,651 SF
STREET FRONTAGE	100 FT	N/A

LEGEND

BCC	BITUMINOUS CONCRETE CURB
EP	EDGE OF PAVEMENT
LF	LINEAR FEET
R	RADIUS
SF	SQUARE FEET
SGC	SLOPED GRANITE CURB
TYP	TYPICAL
☆	EXISTING LIGHT POLE
—	EXISTING SIGN
—	PROPOSED SIGN
● LP	PROPOSED LIGHT POLE BASE
⊙	EXISTING SEWER MANHOLE
⊠	PROPOSED ELECTRIC TRANSFORMER
⊙B	PROPOSED BOLLARD
⊠	PROPOSED ELECTRIC METER
③	PROPOSED PARKING COUNT
—	EXISTING WETLANDS
—	PROVIDE SNOW STORAGE
—	PROPERTY LINE
—	EDGE OF WETLANDS
—	EDGE OF WETLANDS BUFFER
—	PROPOSED VINYL FENCE
—	EXISTING TREELINE
—	PROPOSED TREELINE

ID	SIGN	NO./SIZE/COLOR
A	STOP	R1-1 30" x 30" WHITE LETTERS AND BORDER & RED BACKGROUND



SITE DEVELOPMENT PLANS

TAX MAP H LOT 35-D-0
SITE & LAYOUT PLAN
LILAC GARDEN TRASH COMPACTOR SITE
1 LILAC LANE
DOVER, NEW HAMPSHIRE

OWNED BY
LILAC GARDEN, LLC

SCALE: 1" = 20' (22X34)
1" = 40' (11X17)

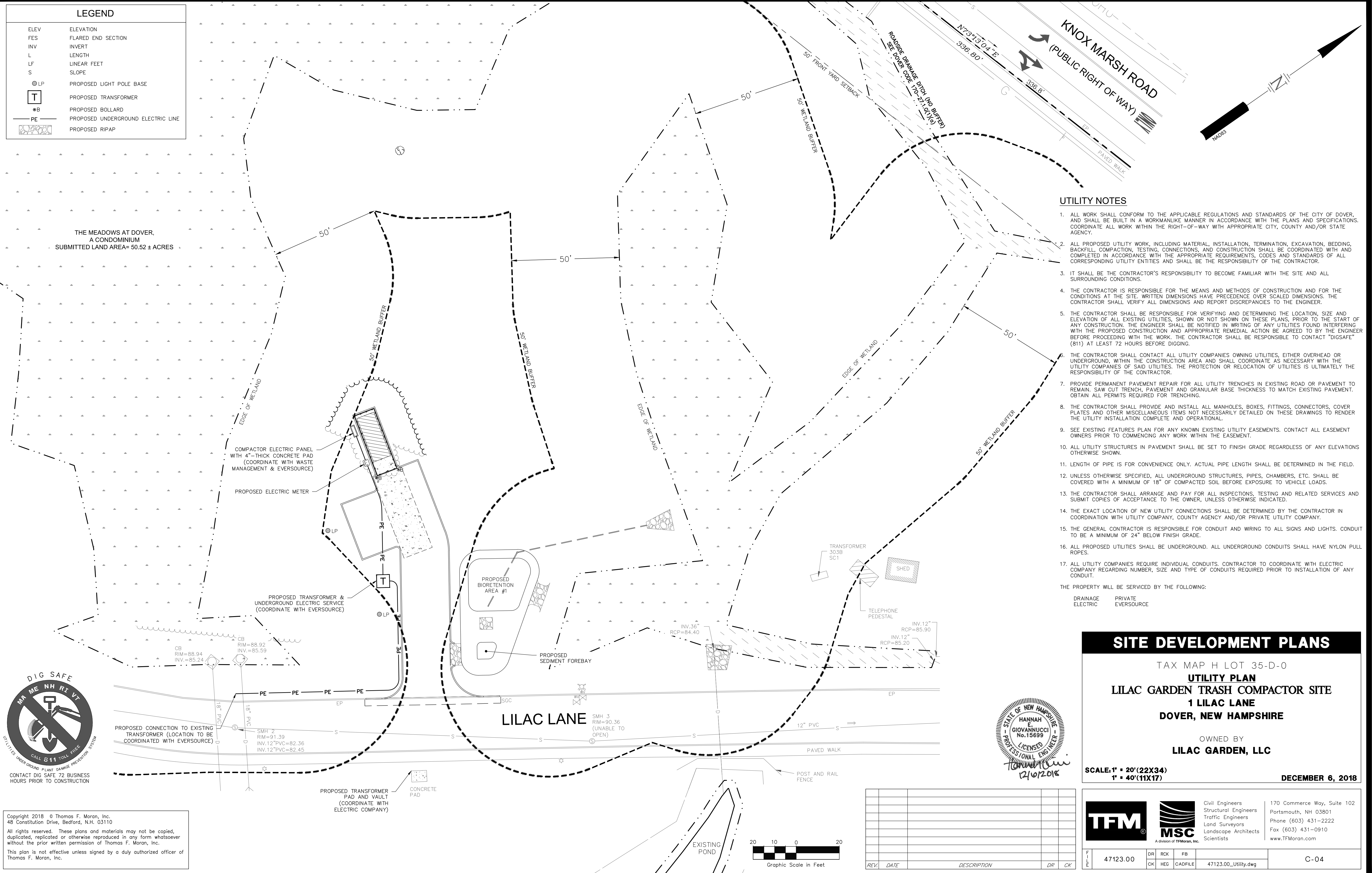
DECEMBER 6, 2018



Civil Engineers
Structural Engineers
Traffic Engineers
Land Surveyors
Landscape Architects
Scientists

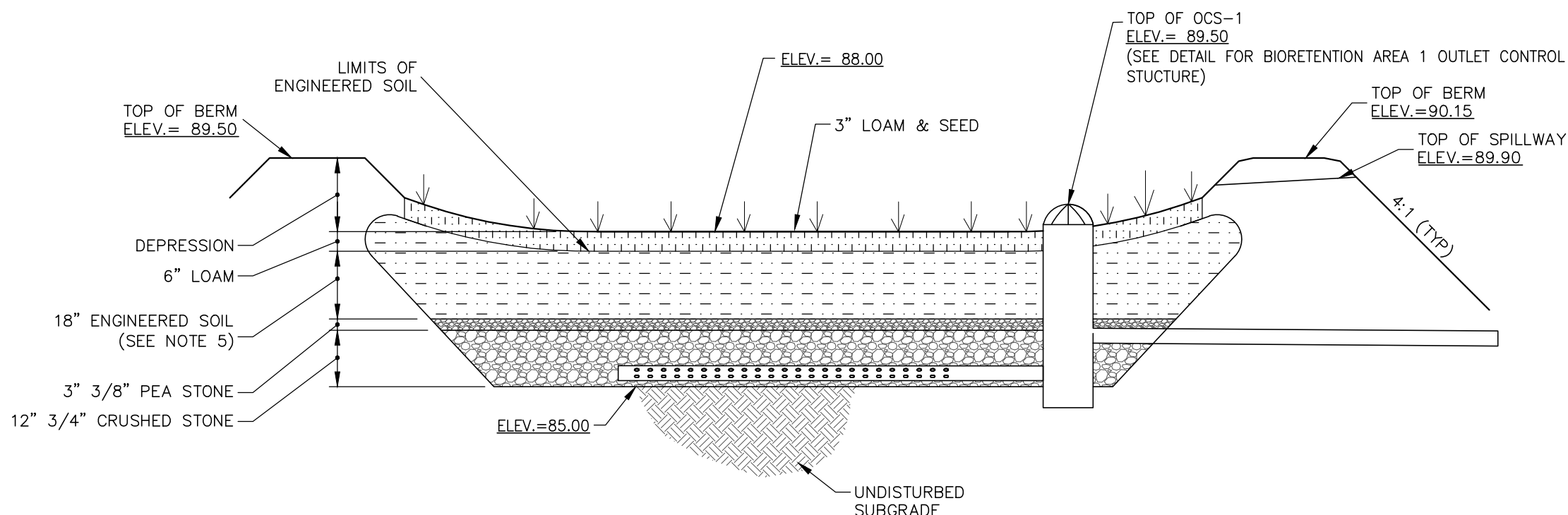
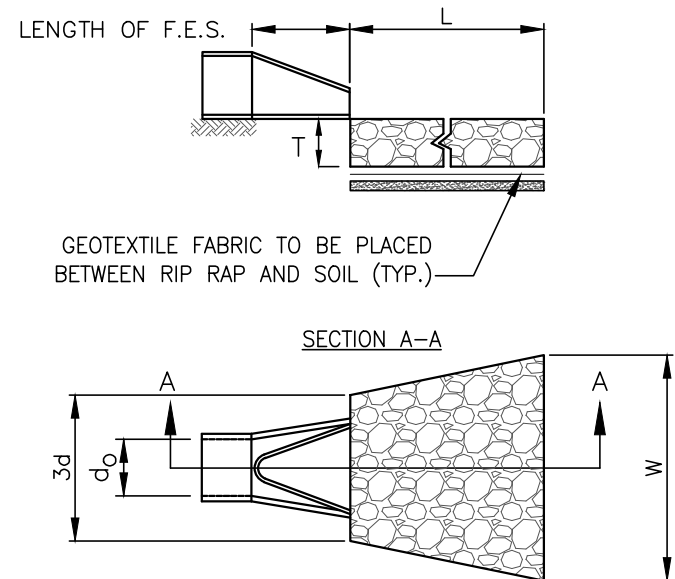
170 Commerce Way, Suite 102
Portsmouth, NH 03801
Phone (603) 431-2222
Fax (603) 431-0910
www.TFMoran.com

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		CK	HEG	CADFILE	47123.00_Site.dwg	C-02



THE OUTLET PROTECTION SHOULD BE CHECKED AT LEAST ANNUALLY AND AFTER EVERY MAJOR STORM. IF THE RIP RAP HAS BEEN DISPLACED, UNDERMINED OR DAMAGED, IT SHOULD BE CHECKED TO SEE THAT EROSION IS NOT OCCURRING. THE DOWNSTREAM CHANNEL SHOULD BE KEPT CLEAR OF OBSTRUCTIONS SUCH AS FALLEN TREES, DEBRIS, AND SEDIMENT THAT COULD CHANGE FLOW PATTERNS AND/OR TAILWATER DEPTHS ON THE PIPES. REPAIRS MUST BE CARRIED OUT IMMEDIATELY TO AVOID ADDITIONAL DAMAGE TO THE OUTLET PROTECTION APRON.

1. THE SUBGRADE FOR THE FILTER MATERIAL, GEOTEXTILE FABRIC, AND RIP SHALL BE PREPARED TO THE LINES AND GRADES SHOWN ON THE PLANS.
2. THE ROCK OR GRAVEL USED FOR FILTER OR RIP SHALL CONFORM TO THE SPECIFIED GRADATION.
3. GEOTEXTILE FABRICS SHALL BE PROTECTED FROM PUNCTURE OR TEARING DURING THE PLACEMENT OF THE ROCK RIP RAP. DAMAGED AREAS IN THE FABRIC SHALL BE REPAIRED BY PLACING A PIECE OF FABRIC OVER THE DAMAGED AREA OR BY COMPLETE REPLACEMENT OF THE FABRIC. ALL OVERLAPS REQUIRED FOR REPAIRS OR JOINING TWO LAYERS OF FABRIC SHALL BE A MINIMUM OF 12".
4. STONE FOR THE RIP RAP MAY BE PLACED BY EQUIPMENT AND SHALL BE CONSTRUCTED TO THE FULL LAYER THICKNESS IN ONE OPERATION AND IN SUCH A MANNER AS TO PREVENT SEGREGATION OF THE STONE SIZES.
5. ADD ANIMAL SCREEN TO FLARED END SECTION OUTLET.

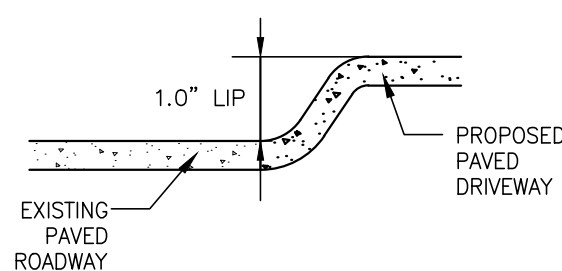


MAINTENANCE SCHEDULE TO BEGIN AFTER CONSTRUCTION IS FINISHED AND BASIN STABILIZATION IS COMPLETE

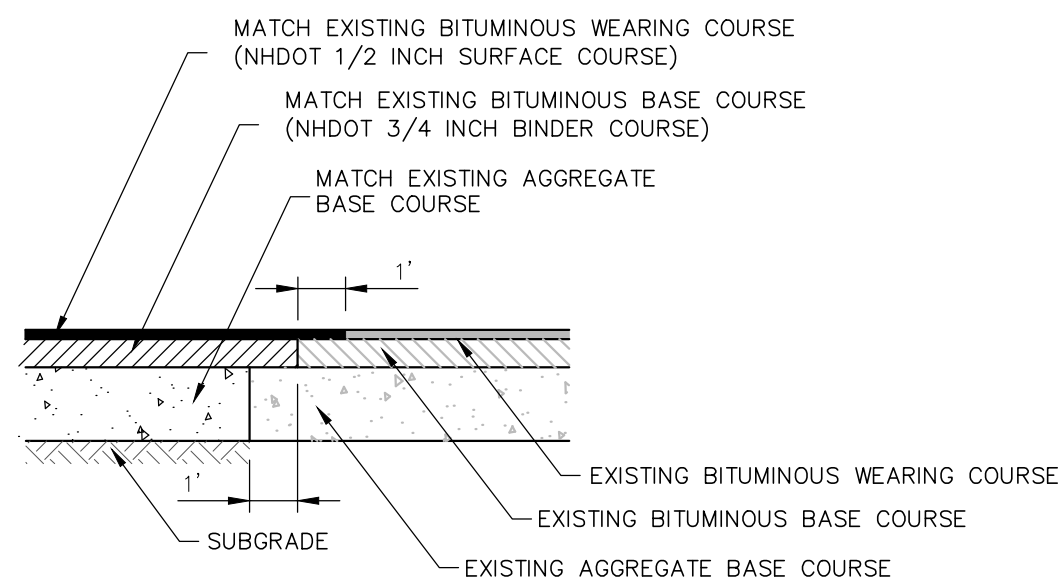
1. VEGETATION - THE VEGETATED AREAS SHALL BE PROTECTED FROM DAMAGE BY TRAFFIC AND DENSE WEED GROWTH. A MINIMUM OF 2 TONS PER ACRE OF AGRICULTURAL LIMESTONE AND 500 LBS. PER ACRE OF 10-20-20 FERTILIZER SHALL BE APPLIED.
2. SEDIMENT - SEDIMENT SHALL BE CONTINUALLY CHECKED IN BIORETENTION AREAS. WHEN SEDIMENT ACCUMULATIONS LINE THE BOTTOM OF THE FACILITY, THEN THE SEDIMENT SHALL BE REMOVED AND PROPERLY DISPOSED OF.
3. PLANTINGS - SHALL BE INSPECTED SEASONALLY TO DETERMINE HEALTH. IF DETERMINED TO BE NOT IN ADEQUATE SHAPE THE PLANTING SHALL BE REMOVED AND REPLANTED WITH THE SAME SPECIES. PROPER MAINTENANCE OF PLANTS SHALL BE CARRIED OUT AS NEEDED.
4. GRASS TO BE TRIMMED TWICE A YEAR.
5. REPAIR ANY ERODED AREA OF THE BIORETENTION AREA BOTTOMS WITH APPROPRIATE LOAM AND SEED AS NECESSARY.

1. USE NEW ENGLAND EROSION CONTROL/RESTORATION MIX BY NEW ENGLAND WETLAND PLANTS, INC. OR EQUIVALENT.
2. SEED AT A RATE OF 1LB/1250SF. APPLY TO BARE SOIL. LIGHTLY MULCH WITH CLEAN WEED FREE STRAW.

NOT TO SCALE



NOT TO SCALE



NOT TO SCALE

NOT TO SCALE



TAX MAP H LOT 35-D-0

LILAC GARDEN TRASH COMPACTOR SITE

DOVER, NEW HAMPSHIRE

OWNED BY

LILAC GARDEN, LLC

SCALE, NTS

DECEMBER 6, 2018



Civil Engineers
Structural Engineers
Traffic Engineers
Land Surveyors
Landscape Architects
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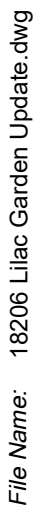
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Dec.06, 2018 - 11:13am
F:\MISC Projects\47123 - Monument Garden\TRASH COMPACTOR\Design\Production Drawings\47123.00 Details.dwg

Dec 06, 2018 - 11:13am



Calculation Summary							
Label	CalcType	Units	Avg	Max	Min	Avg/Min	Max/Min
Site Points	Illuminance	Fc	0.07	5.1	0.0	N.A.	N.A.
Compactor Paved Area	Illuminance	Fc	2.20	5.1	0.5	4.40	10.20



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PROJECT NAME

**Lilac Garden
Trash Compactor
Site- Lilac Lane,
Dover NH**

CLIENT

**MSC a division of
TFMoran, Inc.**
170 Commerce Way, Suite 102
Portsmouth, NH 03801

SEAL

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<i>REVISIONS</i>		
No.	Description	Date
5.		
4.		
3.		
2.		
1.		

Issue Date:

December 4, 2018

Project #:

18206

Drawn:

AD

Checked:

TJ

Approved:

MC

Scale:

1"=15'-0"

DRAWING NAME

**Site Lighting-
Lighting Plan,
Schedules and
Specifications**

DRAWING NUMBER

SL1

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